#### **ADVERTISEMENT FOR RE-BIDS**

Sealed proposals will be received in the Office of the Procurement Manager, Greenville Utilities Commission, 401 S. Greene Street, Greenville, North Carolina 27834 until <u>3:00 pm</u> (EDST) on <u>March 31, 2022</u> and immediately thereafter publicly opened and read for the furnishing of Tubular Steel Structures.

Instructions for submitting bids and complete specifications will be available in the

Office of the Procurement Manager, Greenville Utilities Commission, 401 S. Greene Street,

Greenville, North Carolina during regular office hours, which are 8:30AM - 5:00PM Monday

through Friday.

Greenville Utilities Commission reserves the right to reject any or all bids. Late bids will not be considered.

#### **Notice to Bidders:**

Greenville Utilities Commission is committed to the health and safety of our customers and employees. We are taking the spread of COVID-19 very seriously and continue to monitor the latest Local, State, and Federal guidance.

#### **SECTION I**

#### **GENERAL INSTRUCTIONS FOR FORMAL BIDS**

#### **RELATED TO THE PURCHASE OF APPARATUS, SUPPLIES,**

#### MATERIALS, AND EQUIPMENT

#### 1. NOTICE TO BIDDERS

Sealed bids, subject to the conditions made a part hereof, will be received in the Office of the Procurement Manager, Greenville Utilities Commission, 401 S. Greene Street, Greenville, North Carolina 27834 until <u>3:00 pm</u> (EDST) on <u>March 31, 2022</u>, the day of opening. Bids submitted in a fax or e-mail in response to this Invitation for Bids will not be acceptable.

#### 2. STANDARD FORMS REQUIRED

Each bidder must submit a proposal on the enclosed bid forms. The bid must be signed by an authorized official of the firm. Return only the attached Proposal Form. Do not return the Advertisement for Bids, Instructions to Bidders or Specifications.

#### 3. PREPARATION OF BID

Bids must be in sealed envelopes clearly marked on the outside with the name of the bid and the bid opening date and time. Bid shall be addressed to PROCUREMENT MANAGER, GREENVILLE UTILITIES COMMISSION, 401 S. GREENE STREET, GREENVILLE, NORTH CAROLINA 27834.

#### 4. <u>TIME FOR OPENING BIDS</u>

Bids will be opened promptly and read at the hour and on the date set forth in the advertisement in the Office of the Procurement Manager, Greenville Utilities Main Office, 401 S. Greene Street, Greenville, North Carolina. Bidders or their authorized agents are invited to be present.

#### 5. <u>BID SECURITY</u>

Each Proposal shall be accompanied by cash, cashier's check, or certified check drawn on a bank insured with the Federal Deposit Insurance Corporation or the Savings Association Insurance Fund, payable to the Owner, in an amount not less than five percent (5%) of the total bid as a guarantee that a Purchase Order, if awarded, will be accepted. In lieu thereof, a Bid Bond may be submitted by the Bidder in an amount not less than five percent (5%) of the total bid.

#### 6. <u>NC SALES TAX</u>

Do **not** include NC sales taxes in bid figure; however, Greenville Utilities Commission (GUC) does pay sales tax. Sales tax should be added to the invoice as a separate item.

#### 7. FEDERAL EXCISE TAX

GUC is exempt from Federal Excise Tax and will issue a Federal Exemption Certificate upon request to the successful bidder.

#### 8. EXCEPTIONS TO BE CLEARLY STATED

If bid is not in strict accordance with Section II, "Specifications," bidder must list or note all exceptions **on the Request for Proposal Form**, otherwise, it is fully understood that the successful bidder will furnish equipment and/or materials exactly as specified. GUC reserves the right to accept or reject bids with noted minor deviations from specifications and to determine the lowest responsible, responsive bid from the standpoint of quality, performance, and price.

#### 9. EVALUATION AND AWARD OF BIDS

GUC reserves the right to reject any and all bids, to waive any and all informalities, and to disregard all nonconforming or conditional bids or counter proposals. In evaluating bids, GUC shall consider whether the bids comply with the prescribed requirements, plus all alternates or options requested. GUC reserves the right to include or exclude any option or alternative in GUC's opinion is in GUC's best interests. If a bid is to be awarded, it will be awarded to the lowest responsible, responsive bidder whose evaluation by GUC indicates that the award will be in GUC's best interests. Only firm prices will be considered for award of this bid.

#### 10. PROMPT PAYMENT DISCOUNTS

Bidders are urged to compute all discounts into the price offered. If a prompt payment discount is offered, it may be considered in the award of the contract.

#### 11. NUMERICAL ERRORS

In the case of a discrepancy between a unit price and the extension (the unit price multiplied by the number of units), the unit price governs. In the case where numerical bids are stated both in numbers and in words, the words govern.

#### 12. BID WITHDRAWAL

A bidder must notify GUC in writing of its request to withdraw a bid within seventy-two (72) hours after the bid opening, not including Saturdays, Sundays, or holidays. In order to justify withdrawal, the bidder must demonstrate that a substantial error exists and that the bid was submitted in good faith.

#### 13. MINORITY BUSINESS PARTICIPATION PROGRAM

GUC has adopted an Affirmative Action and Minority and Women Business Enterprise Plan (M/WBE) Program. Firms submitting a proposal are attesting that they also have taken affirmative action to ensure equality of opportunity in all aspects of employment, and to utilize M/WBE suppliers of materials and/or labor.

#### 14. DELIVERY TIME

Delivery time is to be stated and will be considered in the evaluation of bids. Failure by the successful bidder to meet quoted delivery shall be interpreted as non-compliance with these specifications and may be deemed sufficient cause for removal of the manufacturer and/or distributor from our lists as acceptable manufacturers or bidders.

#### 15. DELIVERY

Shipments will be made only upon individual releases from a blanket purchase order issued by GUC in accordance with GUC's current needs. Time is of the essence with respect to all deliveries under this Agreement. Delivery of all equipment, materials, or supplies shall be made Free on Board (FOB) GUC Warehouse, 701 Utility Way, Greenville, North Carolina 27834, unless otherwise specified. The agreed price for such equipment, materials, or supplies shall include all costs of delivery and ownership, and risks of loss shall not be transferred from Provider to GUC until express written acceptance of delivery and inspection by GUC. Delivery hours are between 8:00 AM and 4:30 PM Tuesday-Thursday only. **GUC's purchase order number is to be shown on the packing slip or any related documents.** GUC reserves the right to refuse or return any delivery with no purchase order number or which is damaged. GUC will not be charged a restocking fee for any delivery which is refused or returned.

#### 16. CONTRACT PERIOD

NA

#### 17. MANUFACTURER

Bidder is to specify the manufacturer of items being quoted if applicable.

#### 18. QUANTITIES

Quantities specified are only estimates of GUC's requirements. GUC reserves the right to purchase more or less than the stated quantities at prices indicated in the submitted Proposal Form based on our actual needs.

#### **19.** CONTACT INFORMATION

Questions regarding this bid request should be directed to Cleve Haddock, CLGPO, Procurement Manager, at (252) 551-1533, <u>haddocgc@guc.com</u>. <u>All questions regarding this</u> <u>bid must be received by or before 5:00 pm (EDST) on March 16, 2022.</u>

#### 20. TERMS AND CONDITIONS

The attached Terms and Conditions apply to all purchases made by Greenville Utilities Commission (GUC) and must be considered as part of the bid proposal.

#### SECTION II

#### **GREENVILLE UTILITIES COMMISSION**

#### SPECIFICATIONS FOR GALVANIZED TUBULAR STEEL STRUCTURES

#### MARCH 31, 2022

#### STEEL SINGLE POLE, AND H-FRAME STRUCTURES

#### 1. SCOPE

This specification covers the design, materials, welding, inspection, protective coatings, drawings and delivery of steel transmission single pole, three pole, and H-frame structures. The proposal submitted by the manufacturer shall include field bolts, locknuts, vangs, attachment provisions for arms and/or insulators, anchor bolts, base plates, and other necessary items to make a complete structure. Tangent structure drawings will be provided after selection of vendor. Proposals shall account for 13 thru holes and and 3 thru vangs for tangent structures. Bak and Lca files are provided.

#### 2. **DEFINITIONS:**

Cambering - the fabricating of a slight convex curve in a pole or crossarm

D/t - the ratio of the diameter of a tubular pole to the steel plate thickness

Engineer - a registered or licensed person, who may be a staff employee or an outside consultant, and who provides engineering services. Engineer also includes duly authorized assistants and representatives of the licensed person.

Ground Line - a designated location on the pole where the surface of the ground will be after installation of a direct embedded pole

Load factors (LF) - a multiplier which is applied to each of the vertical, transverse and longitudinal structure loads to obtain an ultimate load

P-delta (P-t1) moment - secondary moment created by the vertical loads acting on the structure when the structure deflects from its unloaded position

Point of Fixity - location on the pole at ground line or below ground line where the maximum moment occurs

Pole twist - Spiral rotation of a pole section relative to the pole end. It is caused by the residual stress in the steel as received from the mill, the clamping force holding the tube shells together and the heat applied during the seam welding process.

Raking - the practice of installing a straight pole out of plumb, or at an inclined angle

w/t - ratio of the width of the pole (flat-to-flat) to the plate thickness

Ultimate load - the maximum design load which includes the appropriate load factor specified

UNC – Unified Coarse Threads

#### 3. CODES AND STANDARDS

Codes, standards, or other documents referred to in this specification shall be considered as part of this specification. The following codes and standards are referenced:

- a. American Society of Civil Engineers (ASCE) Standard, <u>Design of Steel Transmission</u> <u>Pole Structures</u>, Manual 72, latest edition.
- b. American Society for Testing and Materials (ASTM), various standards, latest revision.
- c. American Concrete Institute (ACI), <u>Building Code Requirements for Reinforced</u> <u>Concrete</u>, ACI 318, latest edition.
- d. American Welding Society (AWS), <u>Structural Welding Code</u>, AWS D1.1, latest edition.
- e. American National Standards Institute (ANSI), <u>National Electrical Safety Code</u>, ANSI C2, latest edition.
- f. Society for Protective Coatings (SSPC, formerly Steel Structure Painting Council)/ National Association of Corrosion Engineers (NACE) <u>Surface Preparations</u> <u>Specification</u>, SSPC/NACE SP-6/NACE 3.

# 4. CONFLICT BETWEEN THIS SPECIFICATION, DRAWINGS, AND REFERENCED DOCUMENTS

In the event of conflict between this specification and the above referenced documents, the requirements of this specification shall take precedence. In the case of conflict between several referenced documents, the more stringent requirement shall be followed. If a conflict exits between this specification or the referenced documents and the attached drawings, the attached drawings shall be followed. If clarification is necessary, contact the owner or owner's representative.

#### 5. GENERAL REQUIREMENTS

The design, fabrication, allowable stresses, processes, tolerances, and inspection shall conform to the ASCE Standard, <u>Design of Steel Transmission Pole Structures (Manual</u> <u>72</u>), latest edition, with the following additions and/or exceptions:

- a. <u>Design</u>
  - (1) Pole designs shall be prepared from the attached Pole Replacement Schedule A and configuration drawings (Attachment B of this Specification). Pole designs shall be 12-sided galvanized steel. The H-frame structure cross arm shall be 8-

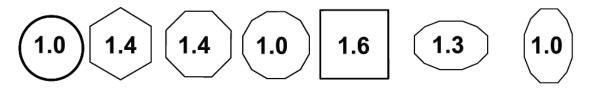
sided galvanized steel. The H-frame structure mounting strap shall be galvanized steel.

(2) Wind pressures shown in the loading criteria shall be multiplied by the appropriate shape factor applied to the poles. Pressures in psf shall be computed as follows:

$$p = W \times C_d$$

Where p = pressure on projected area of the pole normal to wind, W = wind pressure, and  $C_d = shape (or drag) factor.$ 

Shape factors for computing the wind on poles are:



- (3) The maximum design unit stress shall be the minimum yield strength as stated in applicable ASTM specifications for the particular application and types of loads, including load factors.
- (4) Poles shall be designed with a minimum number of joints. Field welding shall not be allowed as part of the design of a new pole. The shaft joints to be made in the field shall be slip joints or bolted flange joints. Slip joint length shall be at least 1-1/2 times the largest inside diameter of the female section. Bolted flange joints shall be used for medium angle and heavy angle guyed structures, cross-braced H-Frames, and structures in uplift.

Manufacturer shall verify slip joint fit before shipment. Joints should not interfere with joints, step nuts, ladder clips, or jacking nuts.

Sufficient jacking lugs and permanent orientation marks shall be provided at all slip joints to ensure proper alignment and complete overlap of the joint.

- (5) The ultimate load in guys shall not exceed 65 percent of the rated breaking strength of the guy.
- (6) Design of anchor bolts shall be in accordance with the ACI-318-1983 Edition, <u>Building Code Requirements for Reinforced Concrete</u>, assuming a concrete strength as specified by the owner.

When anchor bolts are specified, they shall have the top 2 feet galvanized. Anchor bolts shall be threaded at the top end a distance equal to the baseplate thickness plus the thickness of two anchor bolt nuts plus 2-1/2". Each anchor bolt shall include two heavy hex nuts. Welding on anchor bolts will only be allowed in the bottom 12 inches. Only one length of anchor bolt shall be used on each pole. Anchor bolts/clusters shall be plainly marked to indicate the structure type, structure number, orientation, and top of concrete.

Anchor bolts shall be designed to be shipped as a rigid cage with top and bottom plates holding the anchor bolts in place. The anchor bolt thread shall be protected during shipping. The anchor bolts shall be welded to the holding plate in the bottom of the cage. The top template shall be designed to be removable and to support the assembled cage during lifting and setting operations without detrimental deformations. Bolt clusters shall be designed to be rigid enough to withstand the normal jolts of shipping, handling and installation with no displacement of bolts from the proper positions within the cluster.

The removable template at the top shall be marked to show the centerline for tangent structures and the angle bisector for angle structures. Matching marks are to be on the base plate of the structure so proper alignment can be made.

- (7) Minimum plate thickness for all pole components shall be 3/16 inch.
- (8) Structures which are to be direct embedded shall have bearing plates and ground sleeves. Bearing plates shall have a diameter not more than 2 inches greater than the maximum pole diameter.

Galvanized poles shall have a drain hole at the bottom. The drain hole shall not be more than 20% of the bottom plate surface area.

Direct embedded steel poles shall have ground sleeves. Ground sleeves shall have a minimum length of 4 feet centered at ground line.

The ground sleeve shall have a minimum thickness of 3/16 inch and shall be centered at the ground line. A seal weld shall be provided around the ground sleeve. The ground sleeve shall not be considered in strength calculations.

- (9) Poles shall have nearly a uniform taper throughout their entire length. The maximum difference in tapers between two pole sections measured by the diameters shall be .20 inch/ft. for poles with variable taper.
- (10) Poles with elliptical cross sections shall have a minor axis dimension equal to at least 75 percent of the major axis dimension.
- (11) All unguyed angle poles or unguyed tangent deadends shall be precambered to remain plumb when the calculated deflection at the top of the pole exceeds 1.5 percent of the pole height under an initial conductor tension loading of 60°F, no wind, and no load factors. Pole height shall be the height of the pole from the top of the baseplate, or designated ground line, to the top. Tangent poles with unbalanced vertical loadings shall be precambered for the previously stated conditions.

(12) Arms shall be designed so the end of the arm is at the specified height under a loading of initial conductor tension, 60°F, no wind, and no load factors. Arms shall not deflect vertically more than 12 inches at the end of the arm under heavy ice conditions (without any load factors applied).

Arms shall be upswept or straight, tapered, steel tubular members, of any crosssectional type, which meet the dimensions shown on the attached drawings.

Arm end plate connection details for hardware attachment shall be typical of those shown on the attached drawings. The arms shall be hermetically sealed when a painted finish is specified. Galvanized arms shall have drain holes where appropriate.

- (13) Lifting lugs are optional. The manufacturer shall supply all instructions for handling and erection of poles and arms.
- (14) In the design of connections for vangs, brackets, or stiffeners attached to the pole shaft, care shall be taken to distribute the loads sufficiently to protect the wall of the pole from local buckling.
- (15) Thru-vang shall penetrate both sides of the pole with attachment holes on both sides.
- (16) Each pole shall be <u>permanently marked</u> on the pole shaft 60 inches above ground line and on the bottom of baseplate or bearing plate with the following identifying information: manufacturer's identification, structure type, height, structure number, ultimate groundline moment, owner name, and date manufactured. The method of identification shall be approved by the owner.
- (17) Each steel pole section shall be marked with a weld bead indicating the location of that sections center of gravity.
- (18) Each separate part of steel pole structure shall be marked in accordance with SUPPLIER'S drawings. Marking shall be made by addition of weld metal and shall be clearly legible after finishing.
- b. Materials
  - (1) All materials shall comply with the applicable requirements of ASTM specifications. Any modifications to ASTM specifications must be approved by the owner's representative prior to bidding.
  - (2) Poles, arms and conductor brackets shall conform to ASTM A36, ASTM A572, ASTM 581, ASTM A588, ASTM A871 or ASTM A595.
  - (3) Base plate shall conform to ASTM A572, ASTM A588, ASTM A633, or ASTM A595.
  - (4) Anchor bolts shall conform to ASTM A615, Grade 60 or 75.

- (5) Other bolts and nuts shall conform, as applicable, to ASTM A307, ASTM A325, ASTM A354, ASTM A394, or ASTM A687. Locknuts shall be provided for each structure bolt, or American Nut Company (ANCO) type self-locking nuts may be used. Locknuts shall be the galvanized MF type or ANCO type.
- (6) Anchor bolts, structural plate, and weld material, shall meet ASCE requirements for Charpy tests.
- (7) For galvanized structures, steel used for the pole shaft and arms shall have a silicon content less than .06 percent.
- c. Fabrication
  - (1) All welding shall be in accordance with the AWS D1.1, latest edition. Welders shall be qualified in accordance with AWS D1.1 welding procedures.
  - (2) One hundred percent penetration welds shall be required in, but not limited to, the following areas:
    - a. circumferential welds (C-welds) joining structural members;
    - b. longitudinal welds in the female portion of the joint within the slip joint area;
    - c. welds at the butt joints of back-up strips; and
    - d. base plate to shaft weld.
    - e. longitudinal welds for a minimum length of 3 inches where there are adjacent C-welds, flange welds, base welds and ends of tubes.
  - (3) Full penetration or equivalent 90 percent partial penetration with fillet overlay shall be used for arm-to-arm base, vang-to-plate shaft, and arm box joints.
  - (4) Quality and acceptability of every inch of the full penetration welds shall be determined by visual and ultrasonic inspection.
  - (5) All other penetration welds shall have 60 percent minimum penetration. Quality and acceptability of all welds other than full penetration welds shall be determined by visual inspection, supplemented by magnetic particle, ultrasonic or dye penetrant inspection.
  - (6) All weld back-up strips shall be continuous the full length of the welds. Care shall be exercised in the design of welded connections to avoid areas of high stress concentration which could be subject to fatigue or brittle fractures.
  - (7) Field welding shall not be permitted except with owner's approval and the manufacturer's direction in repairing a pole.

- (8) All parts of the structure shall be neatly finished and free from kinks or twists. All holes, blocks, and clips shall be made with sharp tools and shall be cleancut without torn or ragged edges.
- (9) Before being laid out or worked in any manner, structural material shall be straight and clean. If straightening is necessary, it shall be done by methods that will not damage the metal.
- (10) Shearing and cutting shall be performed carefully and all portions of the work shall be finished neatly. Copes and re-entrant cuts shall be filleted before cutting.
- (11) All forming or bending during fabrication shall be done by methods that will prevent embrittlement or loss of strength in the material being worked.
- (12) Holes for connection bolts shall be 1/16 inch larger than the nominal diameter of the bolts. Holes in the flange plates for bolted splices shall be 1/8 inch larger than the bolt diameter. Holes in the base plates for anchor bolts shall be 3/8 inch larger than the nominal diameter of the anchor bolts. The details of all connections and splices shall be subject to the approval of the owner or his representatives.
- (13) Holes in steel plates which are punched must be smooth and cylindrical without excessive tear out or depressions. Any burrs that remain after punching shall be removed by grinding, reaming, etc.
- (14) Holes of any diameter may be drilled in plate of any thickness. Care shall be taken to maintain accuracy when drilling stacks of plates.
- (15) Holes may be made by use of a machine guided oxygen torch. Flame cut edges shall be reasonably smooth and suitable for the stresses transmitted to them.
- (16) Field drilled holes must be approved by the owner. If the manufacturer is aware of the owner's intent to field drill holes, the manufacture must supply a galvanizing touch-up kit for galvanized poles.
- d. Tolerances

Pole Length	<u>One piece</u> : $\pm 2$ inches, or $\pm 1$ inch $\pm 1/8$ inch per 10 feet of length, whichever is greater (i.e 120 foot pole shall have a length of 120 feet $\pm 2\frac{1}{2}$ inches)
	<u>Assembled pole with flange connections</u> : same as for one piece <u>Assembled pole with slip joint connections</u> : The accumulation of the slip joint tolerances not to exceed $-6$ ", $+12$ "
Pole Diameter	-0 inch, +1/4 inch
Pole End Squareness	$\pm 1/2$ inch per foot of pole diameter
Pole Sweep	1/8 inch per 10 feet of pole length

Manufacturing tolerances shall be limited to the following:

Pole Twist Slip Joint tolerances	structure twist shall be limited to plated structures. Connections for account for the pole twist and sh	ot to exceed 4°/tube segment. Overall o 10° for embedded and 6° for base or all appurtenances to the pole shall ould align vertically. ecommendations and total pole length
Location of Groups of Bolt Holes from Top of Pole	±1.0 inches (tolerance to dimension 'A', Figure 2)	A Ç POLE
Location of Centerline Between Groups of Bolt Holes	±1.0 inch (tolerance to dimension 'B', Figure 2)	B
Location of Holes Within a Group of Bolt Holes	±1/8 inch (tolerance to dimension 'C', Figure 2)	FIGURE 2
Bolt Hole Alignment	Not to vary from the longitudina holes by more than 1/16 inch	l pole centerline of that group of
Location of Identification Plate	±2.0 inch	

#### e. Grounding

- (1) A grounding connection shall be welded to the pole shaft 6 inches above the ground collar. The grounding connection will be either the two-hole NEMA pad, or a nut, or a threaded insert installed in the pole, or an approved alternative.
- (2) Grounding pad face shall not be painted or covered with other coatings. The grounding nut thread and grounding pad threads shall be protected from coatings.
- (3) Poles shall be pre-drilled with a nine-sixteenth inch (9/16") hole behind each threaded hole of a two (2)-hole NEMA pad to permit the use of various bolt lengths in completing a grounding connection.
- (4) Threaded inserts installed for grounding shall be made of Type 316 stainless steel and provided with standard ½ inch, 13 UNC threads. Threads shall be protected from coatings.
- f. Finishes
  - (1) The following finishes are acceptable: galvanizing, zinc primer and painting, and below grade coating.

- a. <u>Galvanizing</u> All structures and structural components which are hotdip galvanized shall meet all the requirements of ASTM A123 or ASTM A153. Measures shall be taken to prevent warping and distortion according to ASTM A384 and to prevent embrittlement according to ASTM A143. Poles made of ASTM A588 steel shall not be galvanized due to the high silicon content of the steel. One gallon of zinc enriched paint shall be provided with each five poles.
- b. <u>Coatings for the Embedded Portion of the Pole</u> When poles are to be directly embedded, a 16 mil (minimum dry film thickness), two component hydrocarbon extended polyurethane coating that is resistant to ultraviolet light shall be applied on the exposed surface of the embedded portion of the pole. The coating shall extend from the butt to the top of the ground sleeve. Other coatings shall be approved by the owner prior to their use.
- (2) Bolts and nuts with yield strengths less than 100,000 psi shall be hot-dip galvanized per ASTM A153 and ASTM A143, or mechanically coated with zinc in accordance with ASTM B454, Class 50. Bolting materials with yield strengths in excess of 100,000 psi shall not be hot-dip galvanized. Instead, they shall be painted with zinc enriched paint or mechanically coated with zinc per ASTM B454, Class 50.
- (3) Compliance with coating thickness requirements shall be checked with a magnetic thickness gauge.

#### g. Inspection and Testing

- (1) The owner and the owner's designated agents shall have free entry at all times while work is being carried on, to all parts of the manufacturer's plant to inspect any part of the production of the poles covered by this specification.
- (2) Steel members which are bent or warped or otherwise improperly fabricated shall be properly repaired or replaced.
- (3) The cost of tests made by the manufacturer (except full scale load tests on poles), including cost of the certified test reports shall be considered included in the price.
- (4) The manufacturer shall make tests in accordance with ASTM A370 and ASTM A673verifying that the material used in the structures, meets the impact properties.
- (5) Mill test reports showing chemical and physical properties of all material furnished under this specification shall be maintained by the manufacturer for a period of 5 years and shall be traceable to the structure.
- (6) All plates over 1-1/2 inches thick shall be ultrasonically tested to assure against defects which could lead to lamellar tearing.

- (7) Welders or welding operators shall be qualified in accordance with the provisions of AWS D1.1.
- (8) The manufacturer shall make certified welding reports for each structure. The reports covering welding shall include all welds of each structure. Each weld shall be clearly identified; and the report shall consist of the method of testing, whether the weld is acceptable, the identification of the structure, the date, and the name and signature of the inspector.
- h. Shipping
  - (1) Each shipment shall be accompanied by a list of all parts, identifiable by structure type and number. Arms, bolts and miscellaneous hardware will be identified by the list for match up with the respective pole shaft. All parts required for any one structure shall be in one shipment, if possible.
  - (2) The owner and owner's representative shall be notified prior to shipment that such shipment is to take place, and they reserve the right to inspect the components prior to shipment. The notification shall give quantities, weight, name of common carrier used, and expected time of arrival with at least two (2) working days' notice of delivery. Delivery of all items of material shall be made at such time as to permit unloading between the hours of 9:00 a.m. and 3:00 p.m., Tuesday through Thursday, holidays excluded.
  - (3) The anchor bolts shall be welded to the holding plate in the bottom of the cage. A removable template shall be used at the top of the cage and shall be marked to show the centerline for tangent structures and the angle bisector for angle structures. Matching marks are to be on the base plate so proper alignment can be made. Bolt clusters shall be rigid enough to withstand the normal jolts of shipping and handling with no displacement of bolts from the proper positions within the cluster.
  - (4) Unless otherwise agreed to by the owner, the anchor bolt cage shall be shipped at least 30 days prior to pole shipment.
  - (5) Salt-treated wood blocking and urethane foams shall not be used when shipping or storing steel poles.
  - (6) Delivery shall be made either to a single designated location or to the individual structure locations.
- i. Standard Class Designations
  - (1) Tangent and guyed angle structures have been specified using RUS Standard Steel Pole Class Designations shown in Table 1 unless noted otherwise.
  - (2) Pole designs shall be prepared for the attached Standard Class design loads. The poles shall be designed to meet ASCE Manual No. 48-11, "Design of Steel Transmission Pole Structures," design methods. The point-of-fixity shall be

considered to be located at a distance from the pole bottom that is equal to seven percent (7%) of the pole length.

The pole shall be symmetrically designed such that the strength required in any one direction shall be required in all directions about the longitudinal axis.

- (3) Using the corresponding values in Table 1, the poles shall be designed for the following requirements.
  - a. The pole shall develop the minimum ultimate moment capacity required in Table 1 at a distance of five feet (5'-0'') from the pole top
  - b. The pole shall develop the minimum ultimate moment capacity above the point-of-fixity that is calculated by multiplying the tip load in Table 1 by the distance to the tip load.
  - c. The geometry and taper of the pole shall be uniform throughout their entire length (top to butt).
- (4) The poles shall be designed to withstand the specified tip loading in Table 1 without exceeding a pole deflection of ten percent (10%) of the pole length above the point-of-fixity when tested in accordance with ASCE Manual No. 48-11.
- (5) Overall length of poles shall be designed and manufactured in incremental lengths of five feet (5'-0'').

	8 1	
	Minimum Ultimate	Horizontal Tip
Standard Class	Moment Capacity at	Load Applied 2 ft
Designations	5 ft from Pole Top	from Pole Top
for Steel Poles	(ft. Kips)	(lbs.)
S-20.0	160	20000
S-19.0	152	19000
S-18.0	144	18000
S-17.0	136	17000
S-16.0	128	16000
S-15.0	120	15000
S-14.0	112	14000
S-13.0	104	13000
S-12.0	96	12000
S-11.0	88	11000
S-10.0	80	10000
S-09.0	72	9000
S-08.0	64	8000
S-07.4	57	7410
S-06.5	50	6500
S-05.7	44	5655
S-04.9	38	4875

**TABLE 1 – Strength Requirements** 

S-04.2	32	4160
S-03.5	27	3510
S-02.9	23	2925
S-02.4	19	2405
S-02.0	15	1950

#### 6. INFORMATION TO BE SUPPLIED BY THE MANUFACTURER

#### a. Information to be supplied with the proposal (Attachment D of this Specification).

- (1) Calculated shipping weight of each structure excluding anchor bolts. Separate weights shall be given for arms and poles.
- (2) Calculated shipping weight of anchor bolts.
- (3) Ultimate ground line reactions (including load factors) in poles and guy wires.
- (4) Anchor bolt size, length and locations (bolt circle diameters).
- (5) Type of material of major components (ASTM number).
- (6) Description of pole shaft, including thickness, length, diameter, crosssectional geometry, and method of fastening each shaft component.
- (7) Data showing the design of the arm, arm connections, arm attachment plates and brackets.
- (8) Sketches or draft drawings of structure and structure attachments.
- (9) Diameter, Length, and Thickness of each vibratory caisson.

#### b. Documentation to be supplied for the owner's approval prior to fabrication

Documentation includes final design calculations for pole shaft, base plate, anchor bolts, arms, and other appurtenances, including their connections for all structures. The following information shall be supplied:

- (1) For the loading cases with load factors, the total shear, axial forces, moments, stresses or stress ratios, section moduli, cross-sectional areas, deflections w/t's for polygonal and D/t's for round cross sections at all splices, at arm attachment points (top and bottom), and at least every 10 feet along the pole.
- (2) For the critical loading case, shear and axial forces, moments, stresses, section moduli, cross-sectional areas at the arm connections, bolt stresses in the arm connection, and deflection at the end of the arm.
- (3) Anticipated deflections at the top of the pole and at the ends of the arms shall be indicated for each pole for the normal, everyday loading condition of 60°F, no wind, and no load factors.

- (4) For all specified loading cases, reactions and ground line moments shall be supplied.
- (5) Detail drawings for each structure type giving weights of structure components, dimensions, and bill of materials.
- (6) Assembly instructions and erection drawings. Slip joint lengths and allowable tolerances. Special handling instructions.
- c. Final Documents shall be supplied to the owner for the items in Section 6.b.(5), after erection of all structures and prior to final payment.
- d. Test Reports (as requested).
  - (1) Certified mill test reports for all structural material.
  - (2) Certified welding reports for each structure.
  - (3) Impact property test reports showing that the material used in the structures meets the impact properties.
  - (4) Test reports on coating thickness.
  - (5) Report of structure testing, when required, including photographs, diagrams, load trees, etc.
  - (6) Material, workmanship, inspection travelers, and material certified mill test reports shall be maintained on file for a minimum of six (6) years by the Materialman, and shall be made available to Greenville Utilities Commission or the Engineer upon request at no charge.

#### 7. APPROVAL, ACCEPTANCE, AND OWNERSHIP

- a. Final designs must be approved by the owner or owner's representative before material ordering and fabrication. Material ordering and fabrication prior to approval will be at supplier's risk. It is understood that award of this contract does not constitute acceptance of design calculations submitted with the bid, if corrections are required in the final structure designs due to manufacturer's errors, omissions, or misinterpretations of the specifications, the quoted price shall not change. Approval of the drawings and calculations by the owner or the owner's representative does not relieve the supplier of responsibility for the adequacy of the design, correctness of dimensions, details on the drawings, and the proper fit of parts.
- b. After delivery, the poles will be inspected and shall be free of dirt, oil blisters, flux, black spots, dross, tear-drop edges, flaking paint or zinc; and in general, shall be smooth, attractive, and unscarred. Poles not meeting this requirement shall be repaired or replaced by the fabricator at no additional cost to the owner.
- c. All final drawings shall become the property of the owner, who shall have full rights to reproduce drawings and use them as the owner sees fit, including submitting them to other vendors for the purpose of obtaining bids on future steel pole purchases.

#### 8. LIST OF ATTACHMENTS TO THIS SPECIFICATION

- a. Attachment A, Pole Replacement Schedule
- b. Attachment B, Structure Drawings
- c. Attachment C, Structure Weights and Costs
- d. Attachment D, Bid Summary-Design Information, and Weights
- e. Attachment E, Previous Questions and Answers

### Attachment A, Pole Replacement Schedule

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			IVIT. PIE	asant - Sugg Pkwy Transmis		<b>.</b>	
Structure #	STR Height	.) Equivalent	<b>RUS Class</b>	Embedment	Embedment	Total Pole	Prelim Embed.
	(ft.)				Depth (ft.)	Length (ft.)	Diameter (in.)
1	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
2	85	Н9	S-10.0	Vibratory Steel Caisson	25	110	30
3	85	ENG	ENG	Concrete Foundation	N/A	85	N/A
4	90	ENG	ENG	Concrete Foundation	N/A	90	N/A
5	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
6	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
7	80	H9	S-10.0	Vibratory Steel Caisson	25	105	30
8	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
9	75	H8	S-09.0	Vibratory Steel Caisson	24	99	28
10	75	H8	S-09.0	Vibratory Steel Caisson	25	100	30
11	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
12	75	H7	S-08.0	Vibratory Steel Caisson	24	99	28
13	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
14	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
15	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
16	75	H8	S-09.0	Vibratory Steel Caisson	25	100	30
17	75	H7	S-08.0	Vibratory Steel Caisson	24	99	28
18	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
19	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
20	80	Н9	S-10.0	Vibratory Steel Caisson	25	105	30
21	90	ENG	ENG	Concrete Foundation	N/A	90	N/A
22	100	ENG	ENG	Concrete Foundation	N/A	100	N/A
23	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
24	80	H8	S-09.0	Vibratory Steel Caisson	25	105	30
25	70	ENG	ENG	Concrete Foundation	N/A	70	N/A
26	75	H10	S-11.0	Vibratory Steel Caisson	25	100	30
27	75	H8	S-09.0	Vibratory Steel Caisson	25	100	30
28	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
29	80	H9	S-10.0	Vibratory Steel Caisson	25	105	30
30	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
31	75	H8	S-09.0	Vibratory Steel Caisson	25	100	30
32	75	H9	S-10.0	Vibratory Steel Caisson	25	100	30
33	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
34	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
35	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
36	75	H8	S-09.0	Vibratory Steel Caisson	24	99	28
37	80	H10	S-11.0	Vibratory Steel Caisson	24	104	28
38	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
39	75	H7	S-08.0	Vibratory Steel Caisson	24	99	28
40	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
40	80	H7	S-08.0	Vibratory Steel Caisson	24	104	28
41	85	ENG	ENG	Concrete Foundation	N/A	85	28 N/A

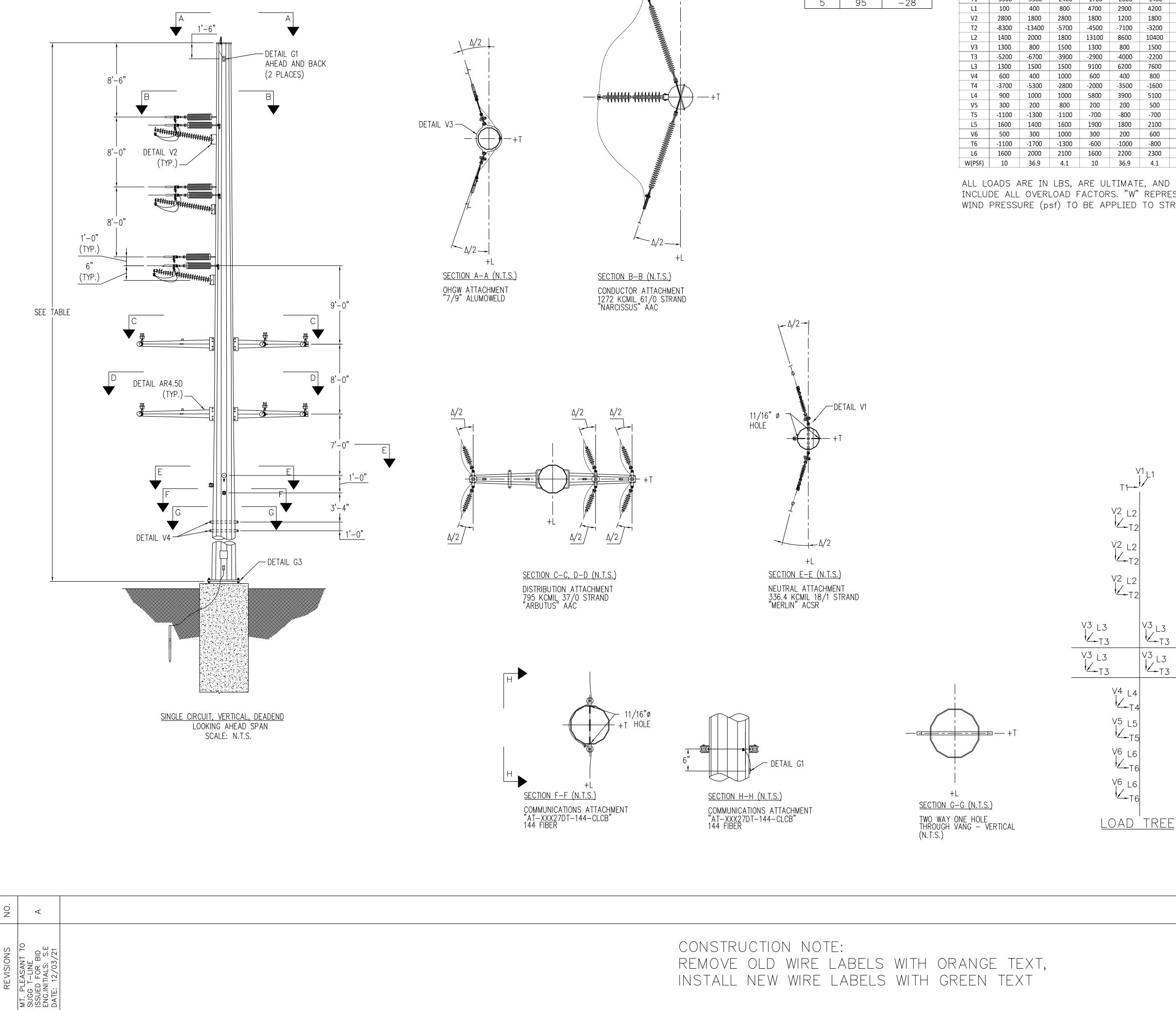
Structure #	STR Height	Wood Pole		Embodmont	Embedment	Total Pole	Prelim Embed.
Structure #	(ft.)	Equivalent	RUS Class	Embedment	Depth (ft.)	Length (ft.)	Diameter (in.)
43	85	H10	S-11.0	Vibratory Steel Caisson	25	110	33
44	80	H8	S-09.0	Vibratory Steel Caisson	25	105	30
45	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
46	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
47	85	H10	S-11.0	Vibratory Steel Caisson	25	110	30
48	90	H10	S-11.0	Vibratory Steel Caisson	25	115	30
49	85	ENG	ENG	Concrete Foundation	N/A	85	N/A
50	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
51	95	H10	S-11.0	Vibratory Steel Caisson	25	120	33
52	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
53	90	H10	S-11.0	Vibratory Steel Caisson	25	115	30
54	90	H10	S-11.0	Vibratory Steel Caisson	25	FALSE	30
55	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
56	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
57	85	H10	S-11.0	Vibratory Steel Caisson	25	110	30
58	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
59	80	H9	S-10.0	Vibratory Steel Caisson	25	105	30
60	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
61	90	ENG	ENG	Concrete Foundation	N/A	90	N/A
62	105	ENG	ENG	Concrete Foundation	N/A	105	N/A
63	105	ENG	ENG	Concrete Foundation	N/A	105	N/A
64	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
65	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
66	70	H7	S-08.0	Vibratory Steel Caisson	24	94	28
67	70	H7	S-08.0	Vibratory Steel Caisson	24	94	28
68	70	H10+	S-11.0+	Vibratory Steel Caisson	24	94	28
69	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
70	70	H8	S-09.0	Vibratory Steel Caisson	24	94	28
71	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
72	70	H9	S-10.0	Vibratory Steel Caisson	25	95	30
73	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
74	80	H10	S-11.0	Vibratory Steel Caisson	25	105	30
75	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
76	80	H8	S-09.0	Vibratory Steel Caisson	25	105	30
77	80	H8	S-09.0	Vibratory Steel Caisson	25	105	30
78	80	H7	S-08.0	Vibratory Steel Caisson	24	104	28
79	80	H9	S-10.0	Vibratory Steel Caisson	25	105	30
80	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
81	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
82	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
83	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
84	85	H8	S-09.0	Vibratory Steel Caisson	24	109	28
85	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
86	75	H7	S-08.0	Vibratory Steel Caisson	24	99	28

Structure #	STR Height	Wood Pole	RUS Class	Embodmont	Embedment	Total Pole	Prelim Embed.
Structure #	(ft.)	Equivalent	RUS Class	Embedment	Depth (ft.)	Length (ft.)	Diameter (in.)
87	75	H6	S-07.4	Vibratory Steel Caisson	24	99	28
88	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
89	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
90	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
91	75	H10	S-11.0	Vibratory Steel Caisson	24	99	28
92	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
93	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
94	75	H10	S-11.0	Vibratory Steel Caisson	24	99	28
95	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
96	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
97	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
98	75	H7	S-08.0	Vibratory Steel Caisson	24	99	28
99	85	ENG	ENG	Concrete Foundation	N/A	85	N/A
100	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
101	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
102	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
103	75	H8	S-09.0	Vibratory Steel Caisson	24	99	28
104	70	H8	S-09.0	Vibratory Steel Caisson	24	94	28
105	70	H4	S-05.7	Vibratory Steel Caisson	24	94	28
106	70	H4	S-05.7	Vibratory Steel Caisson	24	94	28
107	70	H4	S-05.7	Vibratory Steel Caisson	24	94	28
108	70	H5	S-06.5	Vibratory Steel Caisson	24	94	28
109	70	H5	S-06.5	Vibratory Steel Caisson	24	94	28
110	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
111	75	H8	S-09.0	Vibratory Steel Caisson	24	99	28
112	80	H10	S-11.0	Vibratory Steel Caisson	24	104	28
113	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
114	90	ENG	ENG	Concrete Foundation	N/A	90	N/A
115	90	ENG	ENG	Concrete Foundation	N/A	90	N/A
116	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
117	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
118	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
119	90	H10	S-11.0	Vibratory Steel Caisson	24	114	28
120	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
121	90	H10	S-11.0	Vibratory Steel Caisson	25	115	30
122	85	H8	S-09.0	Vibratory Steel Caisson	24	109	28
123	80	H8	S-09.0	Vibratory Steel Caisson	25	105	30
124	80	H8	S-09.0	Vibratory Steel Caisson	24	104	28
125	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
126	85	H7	S-08.0	Vibratory Steel Caisson	25	110	30
127	85	H8	S-09.0	Vibratory Steel Caisson	25	110	30
128	85	H8	S-09.0	Vibratory Steel Caisson	25	110	30
129	85	H8	S-09.0	Vibratory Steel Caisson	25	110	30
130	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28

Structure #	STR Height	Wood Pole	RUS Class	Embedment	Embedment	Total Pole	Prelim Embed.
Structure #	(ft.)	Equivalent	NOS Class	Embedment	Depth (ft.)	Length (ft.)	Diameter (in.)
131	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
132	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
133	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
134	85	H10	S-11.0	Vibratory Steel Caisson	24	109	28
135	85	H7	S-08.0	Vibratory Steel Caisson	24	109	28
136	85	H8	S-09.0	Vibratory Steel Caisson	24	109	28
137	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
138	85	H10	S-11.0	Vibratory Steel Caisson	24	109	28
139	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
140	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
141	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
142	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
143	85	ENG	ENG	Concrete Foundation	N/A	85	N/A
144	85	H10	S-11.0	Vibratory Steel Caisson	25	110	30
145	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
146	75	H6	S-07.4	Vibratory Steel Caisson	24	99	28
147	75	ENG	ENG	Concrete Foundation	N/A	75	N/A
148	75	H6	S-07.4	Vibratory Steel Caisson	24	99	28
149	75	H6	S-07.4	Vibratory Steel Caisson	24	99	28
150	80	H6	S-07.4	Vibratory Steel Caisson	24	104	28
151	80	ENG	ENG	Concrete Foundation	N/A	80	N/A
152	85	H6	S-07.4	Vibratory Steel Caisson	24	109	28
153	85	ENG	ENG	Concrete Foundation	N/A	85	N/A
154	95	ENG	ENG	Concrete Foundation	N/A	95	N/A
155	85	H8	S-09.0	Vibratory Steel Caisson	24	109	28
156	85	H6	S-07.4	Vibratory Steel Caisson	24	109	28
157	80	H7	S-08.0	Vibratory Steel Caisson	24	104	28
158	75	H10	S-11.0	Vibratory Steel Caisson	25	100	30
159	75	H8	S-09.0	Vibratory Steel Caisson	24	99	28
160	70	H7	S-08.0	Vibratory Steel Caisson	24	94	28
161	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
162	70	H6	S-07.4	Vibratory Steel Caisson	24	94	28
163	70	H4	S-05.7	Vibratory Steel Caisson	24	94	28

#### Attachment B, Structure Drawings

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STR #	LENGTH (FT)	ANGLE A
1	80	-14
5	95	-28

			LC	ADING TAE	BLE			
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10
V1	1000	800	1300	700	600	900	600	1400
T1	-3300	-5300	-2400	-1700	-2800	-1400	-800	-2500
L1	100	400	800	4700	2900	4200	100	100
V2	2800	1800	2800	1800	1200	1800	900	3600
Т2	-8300	-13400	-5700	-4500	-7100	-3200	-1400	-7300
L2	1400	2000	1800	13100	8600	10400	700	400
V3	1300	800	1500	1300	800	1500	400	5500
Т3	-5200	-6700	-3900	-2900	-4000	-2200	-900	-4400
L3	1300	1500	1500	9100	6200	7600	600	1400
V4	600	400	1000	600	400	800	200	700
T4	-3700	-5300	-2800	-2000	-3500	-1600	-600	-3000
L4	900	1000	1000	5800	3900	5100	300	300
V5	300	200	800	200	200	500	200	200
T5	-1100	-1300	-1100	-700	-800	-700	-300	-600
L5	1600	1400	1600	1900	1800	2100	700	1100
V6	500	300	1000	300	200	600	200	300
Т6	-1100	-1700	-1300	-600	-1000	-800	-200	-400
L6	1600	2000	2100	1600	2200	2300	400	700
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

ALL LOADS ARE IN LBS, ARE ULTIMATE INCLUDE ALL OVERLOAD FACTORS. "W" WIND PRESSURE (psf) TO BE APPLIED

	CASES
LUAD	UASES

Е,	A	ND	
R	EF	PRESENTS	
ΤC	)	STRUCTURE.	

V1 L1

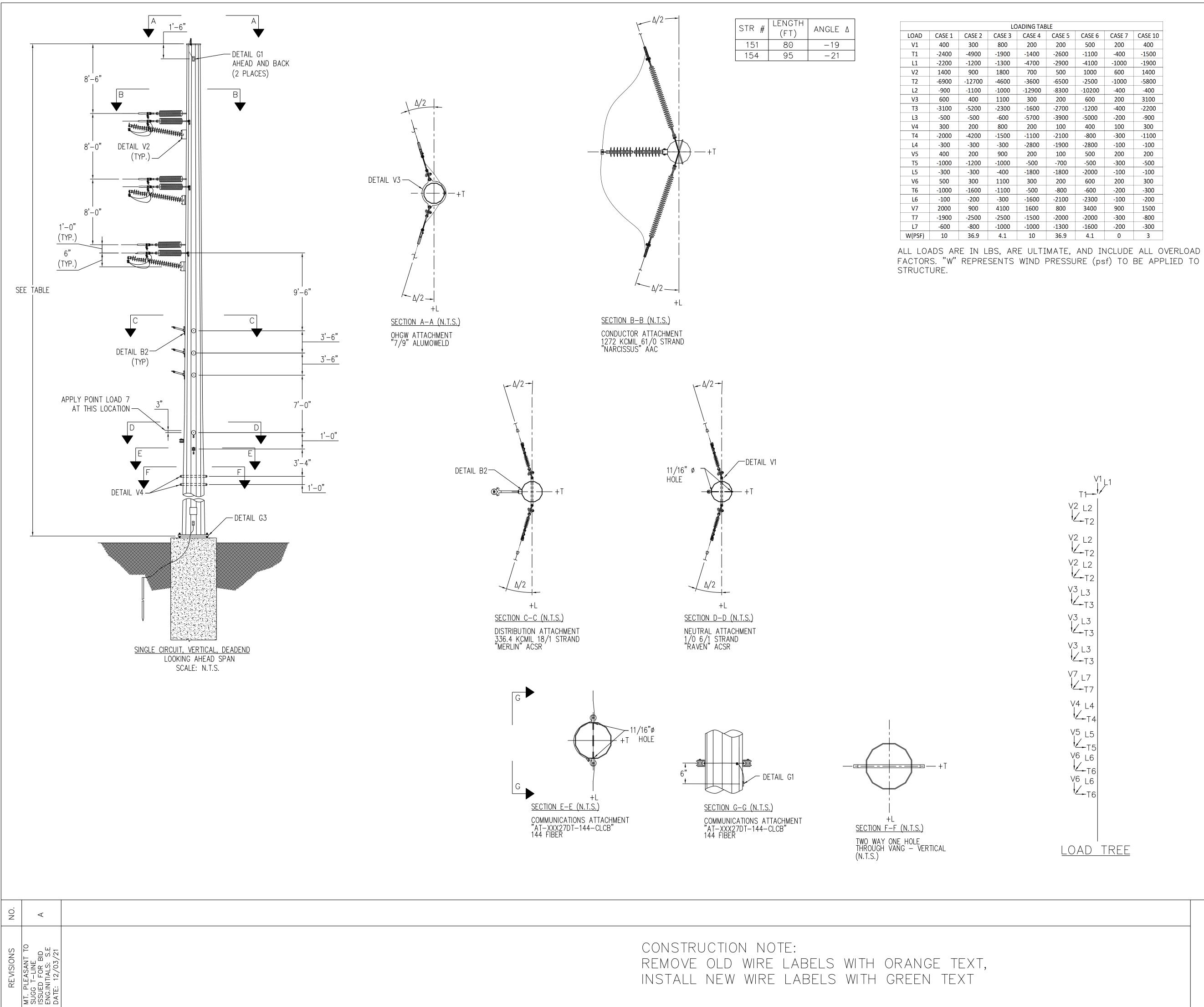
 $\begin{array}{cccc} V_{3} & V_{3} & V_{3} \\ V_{-T3} & V_{-T3} \end{array}$ 

 $\begin{array}{cccc} V_{3}^{3} L_{3} & V_{-13}^{3} \\ V_{-13} & V_{-13}^{3} \end{array}$ 

LOAD (	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
WIRE DA	
OHGW: "7;	#9" ALUMOWELD
	72 KCMIL 61/0 STRAND "NARCISSUS" AAC
	795 KCMIL 37/0 STRAND "ARBUTUS"AAC
	FION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR F-XXX27DT-144-CLCB" 144 FIBER
NOTES:	

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT. 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5%
- OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL.
- 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

			Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUFD				115kV TRANSMI IT. PLEASANT S LOAD AND DEND 0°-30° WI	UB TO SUGG
ISSUED	FUK	ЫD		ISS DATE 12/03/21 JGH APPD. S. ECKMAN	DWG. NO. DE-30L_2Darm_1-CD



	1	1	LC	ADING TAE	BLE	1		1
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10
V1	400	300	800	200	200	500	200	400
T1	-2400	-4900	-1900	-1400	-2600	-1100	-400	-1500
L1	-2200	-1200	-1300	-4700	-2900	-4100	-1000	-1900
V2	1400	900	1800	700	500	1000	600	1400
Т2	-6900	-12700	-4600	-3600	-6500	-2500	-1000	-5800
L2	-900	-1100	-1000	-12900	-8300	-10200	-400	-400
V3	600	400	1100	300	200	600	200	3100
Т3	-3100	-5200	-2300	-1600	-2700	-1200	-400	-2200
L3	-500	-500	-600	-5700	-3900	-5000	-200	-900
V4	300	200	800	200	100	400	100	300
T4	-2000	-4200	-1500	-1100	-2100	-800	-300	-1100
L4	-300	-300	-300	-2800	-1900	-2800	-100	-100
V5	400	200	900	200	100	5 <b>0</b> 0	200	200
T5	-1000	-1200	-1000	-500	-700	-500	-300	-500
L5	-300	-300	-400	-1800	-1800	-2000	-100	-100
V6	500	300	1100	300	200	600	200	300
Т6	-1000	-1600	-1100	-500	-800	-600	-200	-300
L6	-100	-200	-300	-1600	-2100	-2300	-100	-200
V7	2000	900	4100	1600	800	3400	900	1500
T7	-1900	-2500	-2500	-1500	-2000	-2000	-300	-800
L7	-600	-800	-1000	-1000	-1300	-1600	-200	-300
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

V<sup>3</sup> L3 ↓∠\_\_\_\_\_\_ V<sup>3</sup> L3 V7 L7 ↓∠\_\_\_\_\_\_ V4 L4

V<sup>1</sup>L1

⊺1---'

V<sup>2</sup> L2

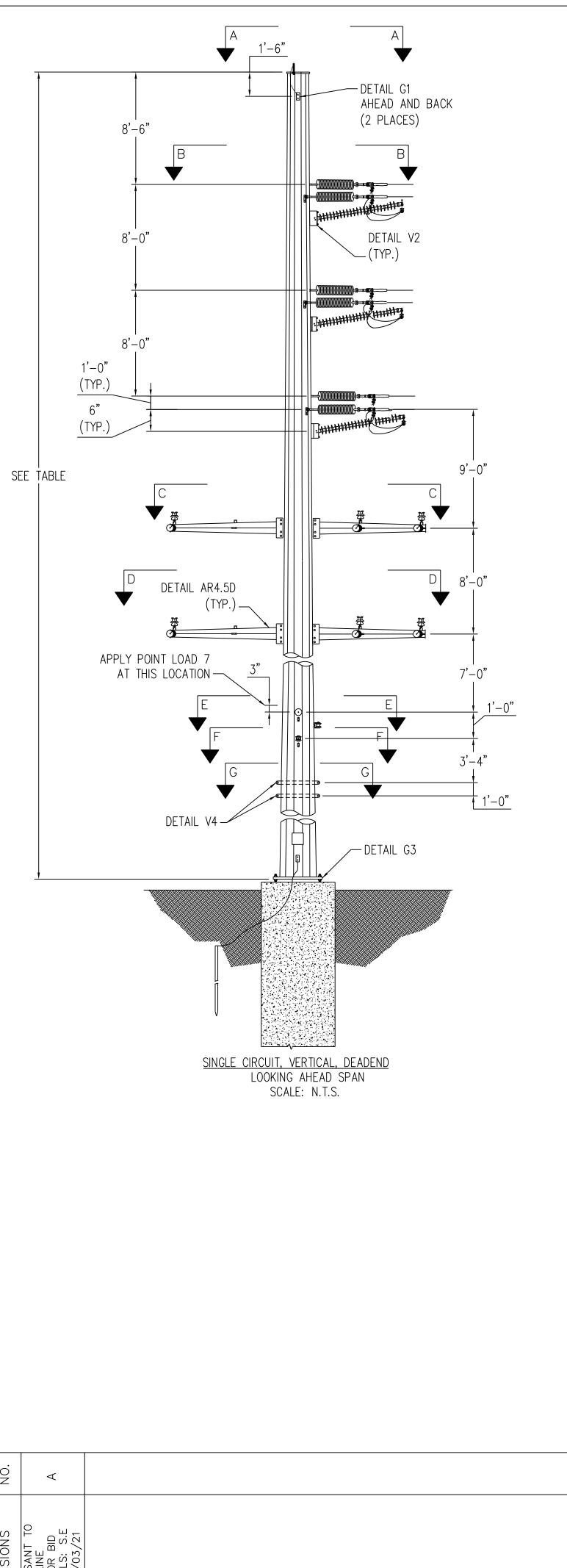
V<sup>2</sup> L2

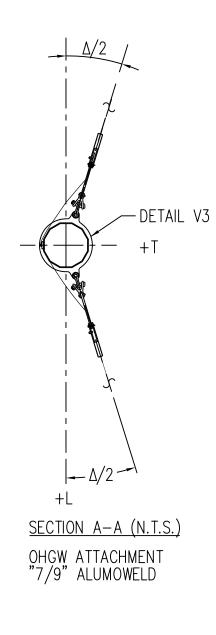
V5 L5 V6 L6 V6 L6 V6 L6 V6 L6 V6 L6

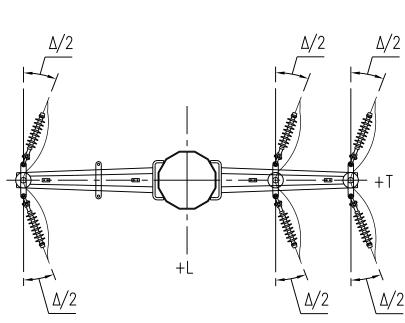
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
WIRE DA	ТА
OHGW: "7	#9" ALUMOWELD
	72 KCMIL 61/0 STRAND "NARCISSUS" AAC
	336.4 KCMIL 18/1 STRAND "MERLIN" ACSR
	FION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR
ADSS: "A	T-XXX27DT-144-CLCB"144 FIBER
NOTES:	
	TATED LOADS ARE ULTIMATE VALUES AND INCLUDE
	OAD FACTORS AND INSULATOR WEIGHT.
	TURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE TANEOUS APPLICATION OF DEAD LOAD, WIND ON THE
	TURE, AND WIRE LOADS FOR EACH LOADING CASE.
	TURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE
	ERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL
· · · · · ·	REMENTS OF THE STEEL POLE SPECIFICATIONS. PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A
	FACTOR OF 1.0.
5. FABRI	CATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT
THAN	THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS

- SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
- DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

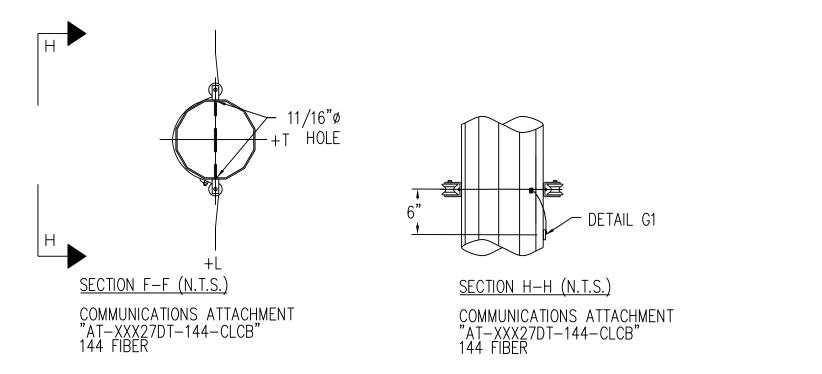
		GREENVILLE Greenville Utilities	
ISSLIED	FOR BID	115kV TRANSM MT. PLEASANT LOAD AND DEADEND 0°-30° V	SUB TO SUGG DESIGN
ISSUED		DWN.D. CHAMBLISS DATE 12/03/21 CKD. R. DILLABOUGH APPD. S. ECKMAN SCALE: NONE	DWG.NO. DE-30L_Vert_1-CD



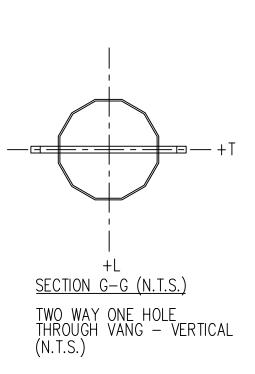


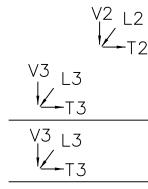


SECTION C-C, D-D (N.T.S.) DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC



NO.	A	
REVI	MT. PLEASANT TO SUGG T-LINE ISSUED FOR BID ENG.INITIALS: S.E DATE: 12/03/21	





V1 L1

V<sup>3</sup>L3 V<sup>3</sup>L3

 $\begin{array}{cccc} V_{3} & V_{3} & V_{3} \\ V_{-T3} & V_{-T3} \end{array}$ 

**₩**\_T3

1 K-T3

| V7 L7

1/\_T7

T1---

V2 L2

 $V_{-T2}$ 

V2 L2

V4 L4I

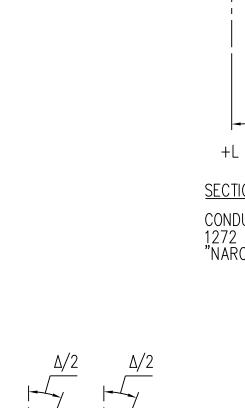
V5 L5

V6 L61

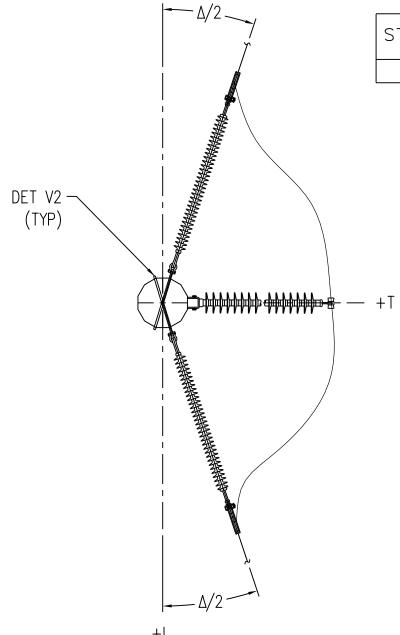
V6 L6

12-T6

LOAD TREE



(TYP)



LENGTH

Δ/2-/

∕\_ 11/16"ø

HOLE

DETAIL V1-

Δ/2\_\_\_\_

+L

SECTION E-E (N.T.S.)

NEUTRAL ATTACHMENT 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

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<u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC

			LC	ADING TAE	IE			
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10
V1	200	100	500	200	100	300	100	100
T1	2200	4700	1700	1300	2500	1000	400	1300
L1	-2200	-1300	-1600	-4800	-3000	-4300	-1000	-2200
V2	800	400	1100	500	300	600	400	600
T2	6000	11900	3900	3400	<mark>6400</mark>	2300	1000	4600
L2	-1700	-2100	-2000	-13300	-8700	-10600	-800	-600
V3	500	200	900	300	200	500	200	4600
Т3	3900	5800	2800	2100	3100	1600	700	3100
L3	-1500	-1600	-1600	-9300	-6200	-7700	-600	-1800
V4	300	200	700	200	200	400	100	200
T4	2800	4700	2000	1500	2500	1200	400	2000
L4	-1000	-1000	-1100	-5900	-4000	-5200	-400	-400
V5	300	100	700	200	100	400	100	100
T5	800	1100	800	500	700	500	300	400
L5	-400	-500	-600	-1900	-1800	-2100	-100	-100
V6	400	200	800	200	100	500	200	200
Т6	900	1400	1000	500	900	600	200	300
L6	-100	-200	-300	-1600	-2100	-2300	-100	-100
V7	700	300	1600	-	-	-	300	400
T7	-200	100	-500	-	-	-	-100	-100
L7	-100	-100	-100	-	-	-	-100	-100
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

## LOAD CASES

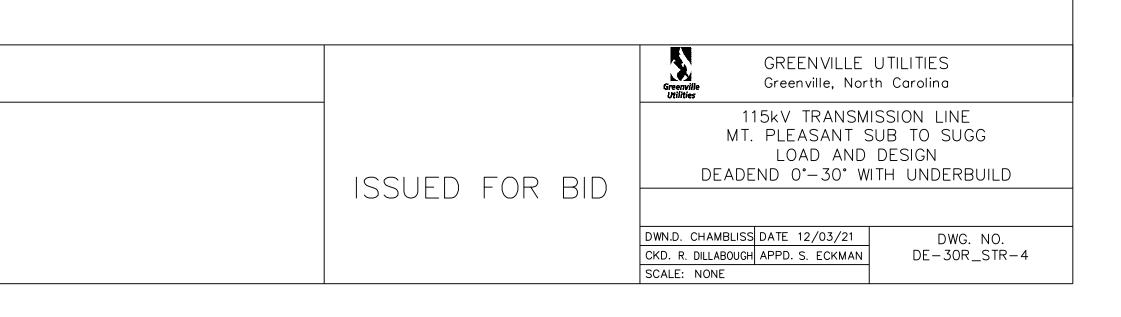
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
<u>wire da</u>	ΓΑ
OHGW: "7 <sub>#</sub>	¥9"ALUMOWELD
	72 KCMIL 61/0 STRAND "NARCISSUS" AAC
	795 KCMIL 37/0 STRAND "ARBUTUS"AAC
DISTRIBUT	ION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

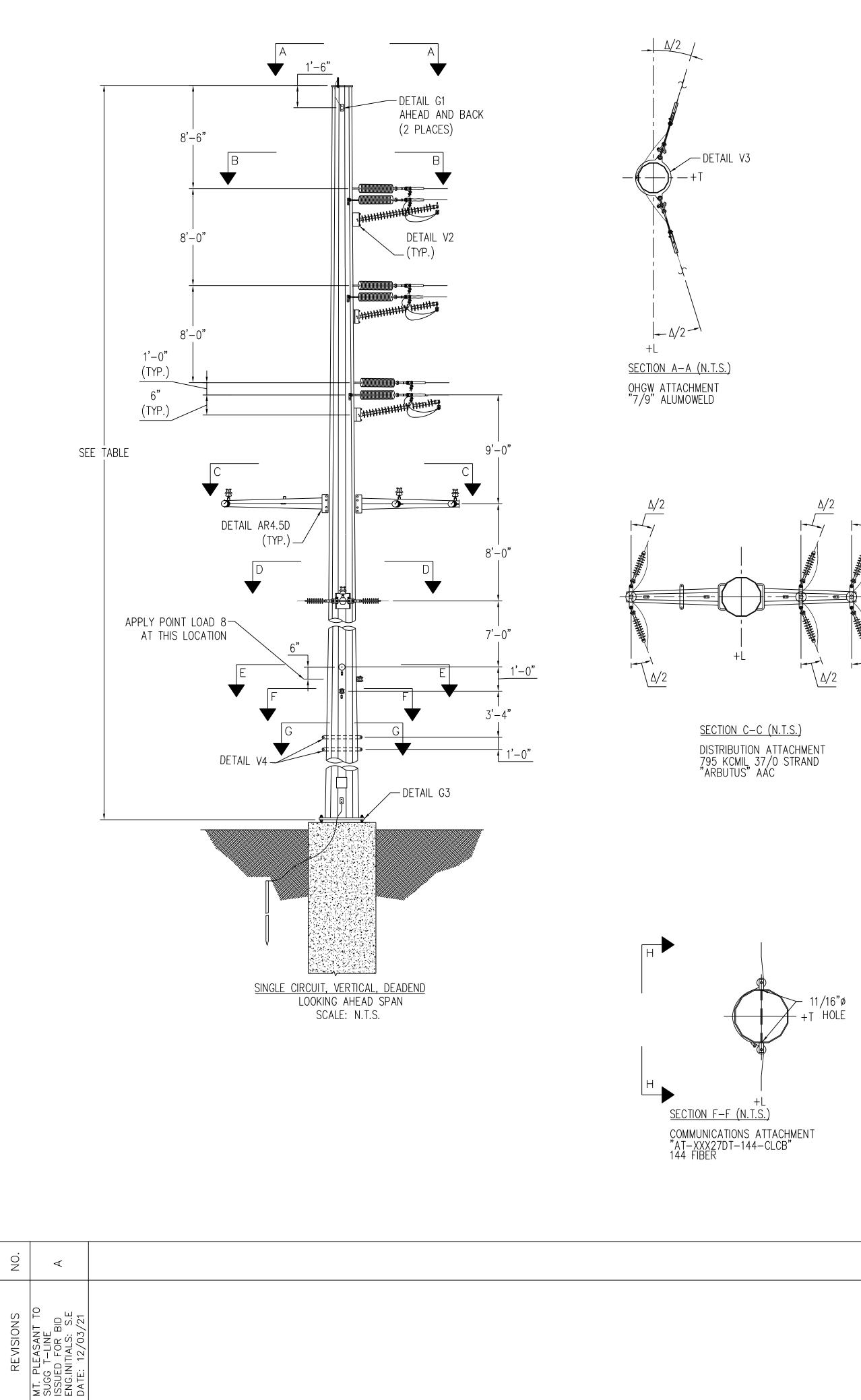
NOTES:

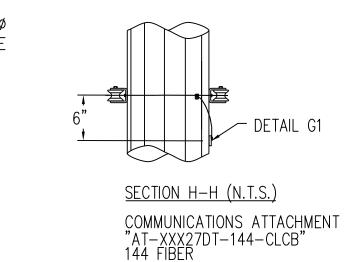
- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".

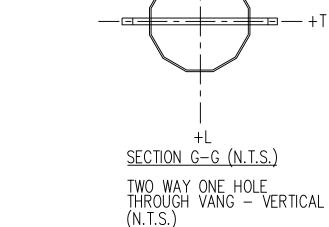
ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

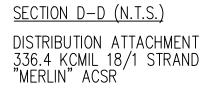
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT. 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5%
- OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
- DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

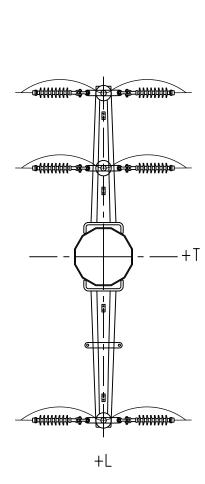












LENGTH

(FT)

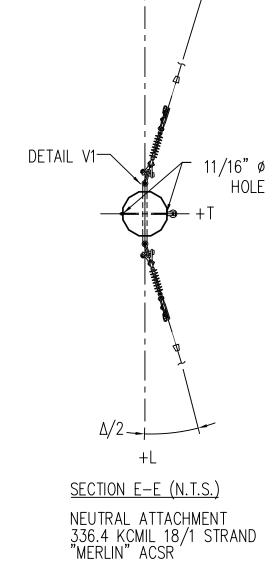
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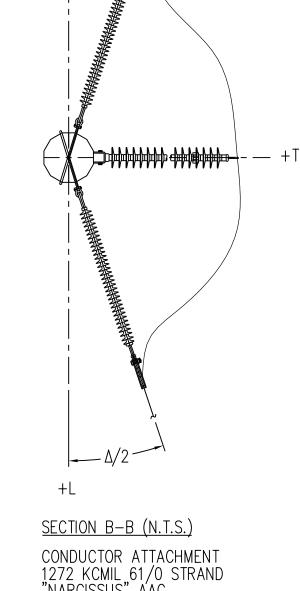
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STR #

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1272 KCMIL 61/0 STRAND "NARCISSUS" AAC

			LC	DADING TAE	BLE			
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10
V1	300	200	700	200	100	500	200	300
T1	2000	4600	1500	1100	2400	800	400	1200
L1	1900	800	600	4800	2800	3700	1100	2500
V2	1200	700	1600	700	400	900	500	1200
Т2	5800	12000	3700	3100	6300	2100	900	4400
L2	900	1000	1100	12700	8000	9900	400	400
V3	800	500	1200	500	300	800	300	4800
Т3	3400	5600	2400	1800	2900	1300	600	2600
L3	800	800	900	8800	5600	7100	300	1500
V4	300	100	600	200	100	500	100	100
T4	2400	4600	1800	1300	2400	1000	300	1700
L4	100	300	500	5100	3500	4700	100	400
V5	200	100	700	200	100	500	100	100
T5	700	1000	700	400	600	400	200	400
L5	200	200	300	1800	1700	1900	-	100
V6	400	100	900	300	100	600	200	200
Т6	800	1400	900	500	800	500	100	300
L6	100	100	200	1600	2100	2200	100	100
V7	2500	1000	6300	1400	600	3500	1000	4100
T7	1300	1900	1400	5200	3600	4300	300	200
L7	200	200	200	900	600	800	100	-100
V8	1400	400	4800	800	300	2700	400	1700
Т8	800	1000	1000	2600	1900	2500	100	-100
L8	100	100	100	500	300	400	100	-100
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

LOAD CASES
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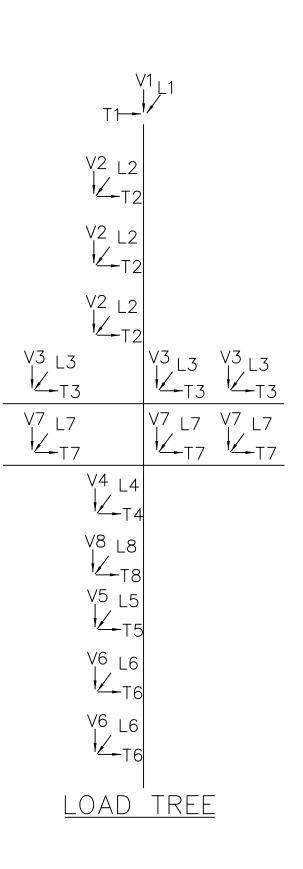
	CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50							
	CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
	CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
	CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50							
	CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
	CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
	CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00							
	CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50							
WIRE DATA									
	OHGW: "7#9" ALUMOWELD								
	115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC								
	12.47kV: 795 KCMIL 37/0 STRAND "ARBUTUS"AAC								
		ROSSING: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR							

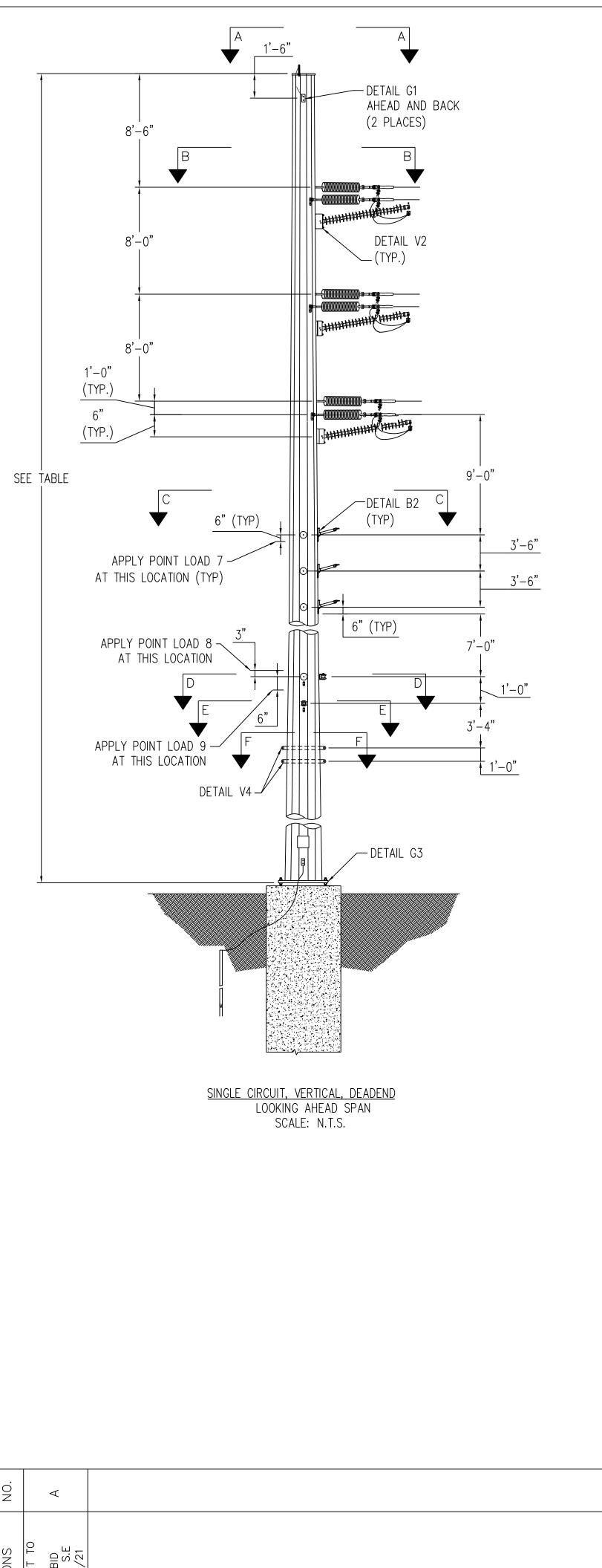
DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL CROSSING: 1/0 6/1 STRAND "RAVEN" ACSR "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

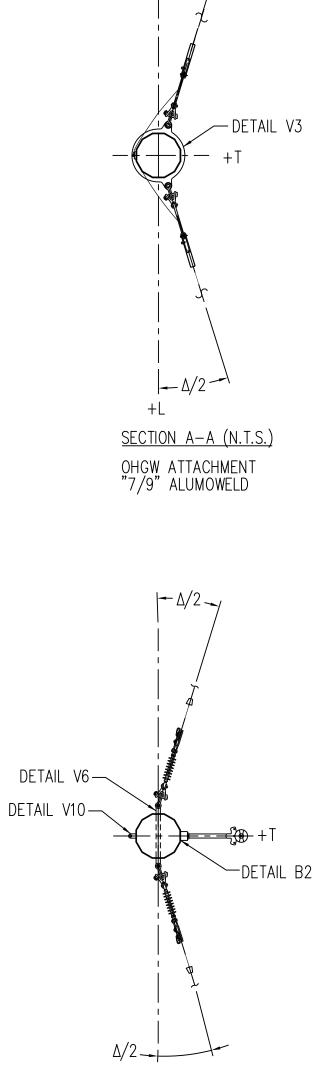
### NOTES:

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE
- STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE. 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL
- REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS. 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

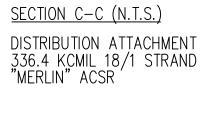
			Greenville Utilities	GREENVILLE U Greenville, North		
	ISSUED FOR BID		115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN DEADEND 0°-30° WITH UNDERBUILD			
		FUR DID		SS DATE 12/03/21	DWG. NO.	
			CKD. R. DILLABOU SCALE: NONE	GH APPD. S. ECKMAN	DE-30R_STR-22	



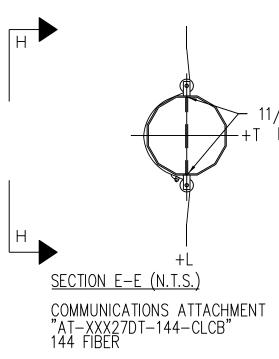




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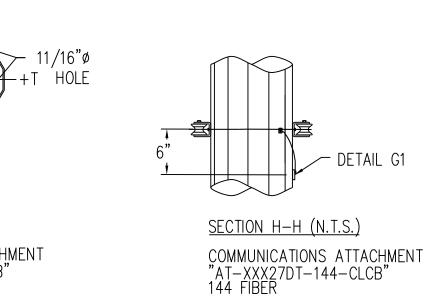


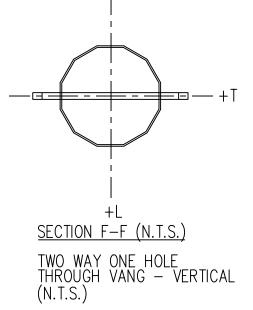
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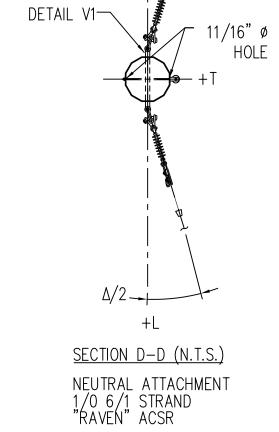
NO.	A	
REVISIONS	MT. PLEASANT TO SUGG T-LINE ISSUED FOR BID ENG.INITIALS: S.E DATE: 12/03/21	

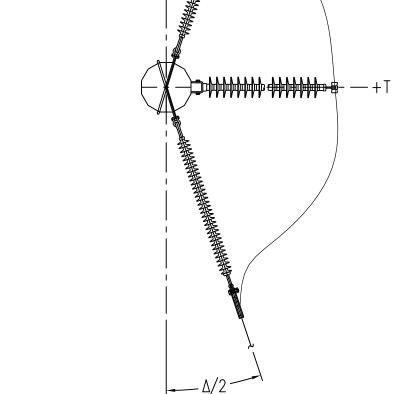
CONSTRUCTION NOTE: REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT











<u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC

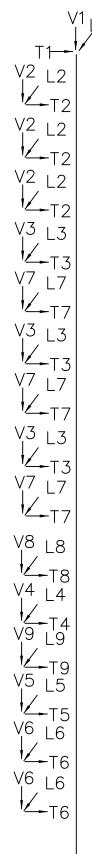
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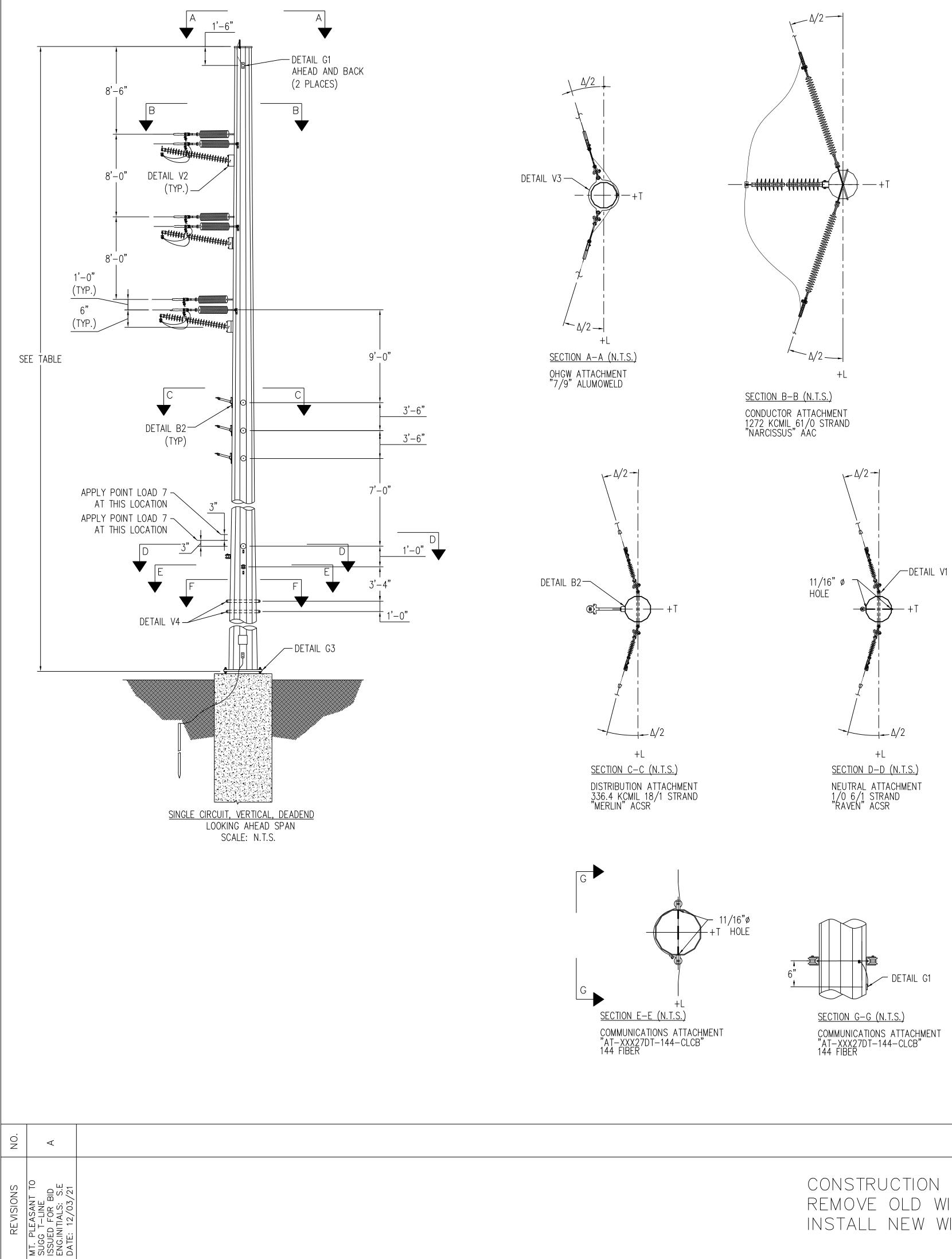
| LENGTH STR # ANGLE 🛛 (FT) 22 64 75

				DADING TAE	21 E				LOAD CASES
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10	CASE 1 NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND
V1	-400	-400	-300	-500	-400 2400	-400	-300	-800	OLF: L=1.65, T=2.50, V=1.50
T1 L1	2300 -2100	4600 -900	1700 -600	1400 -4700	-2600	1000 -3500	500 -1200	1500 -2800	CASE 2 NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND
V2 T2	-1100	-900 11400	-700	-1400	-1000	-1100	-200	-2000	OLF: L=1.00, T=1.00, V=1.00
T2 L2	6400 -1300	11400 -1300	4100 -1300	3300 -12300	5700 -7400	2100 -9400	1000 -500	5400 -700	CASE 3 NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
V3	-500	-500	-300	-500	-500	-500	-100	-1800	CASE 4 NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND
T3 L3	3100 -800	4700 -700	2300 -800	1700 -5400	2500 -3500	1300 -4600	500 -300	2500 -600	OLF: L=1.65, T=2.50, V=1.50
V4 T4	-300 1900	-300 3800	-200 1400	-200 1000	-300 2000	-200 800	-200 300	-500 1200	CASE 5 NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND
L4	-300	-300	-400	-2600	-1700	-2500	-100	-100	OLF: L=1.00, T=1.00, V=1.00
V5 T5	-100 800	-300	-200	-100	-200 500	-200	-100	-200 500	CASE 6 NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND
L5	-200	1000 -300	800 -300	500 -1700	-1600	500 -1800	300 -100	-100	OLF: L=1.00, T=1.00, V=1.00
V6	-100	-400	-200	-100	-300	-200	-100	-100	CASE 7 DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
T6 L6	900 -100	1300 -200	1000 -200	500 -1500	700 -2000	500 -2200	200 -100	300 -100	CASE 10 STRINGING: -20°, 0" ICE, 2 PSF WIND
V7	300	100	900	300	100	1000	100	200	OLF: L=1.50, T=1.50, V=1.50
T7 L7	-2000 -400	-1000 -300	-1600 -400	-2000 -400	-1000 -300	-1600 -400	-600 -200	-2300 -500	WIRE DATA
V8	300	100	900	300	100	1000	100	200	OHGW: "7#9" ALUMOWELD
	-2000 -400	-1000 -300	-1600 -400	-2000 -400	-1000 -300	-1600 -400	-600 -200	-2300 -500	115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC
V9	200	100	800	300	100	800	100	100	12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR
T9 L9	-300	700 -300	600 -300	600 -300	700 -300	600 -300	100 -100	300 -200	ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER
W(PSF)	10	36.9	4.1	-300	36.9	4.1	-100	3	NOTES:
INCL		_ OVER	RLOAD F	FACTOR	S. "W"	REPRES TO STR TO STR V2 L V2 L V2 L V2 L V2 L V3 L V7 L V4 L V5 L V6 L V6 L	V <sup>1</sup> L <sup>1</sup> 2 7 2 7 2 7 2 7 7 7 7 7 7 7 7 7 7 7 7 7		<ul> <li>OVERLOAD FACTORS AND INSULATOR WEIGHT.</li> <li>STRUCTURE, AND ATTACHMENTS SHALL BE DESIGNED FOR EACH LOADING CASE.</li> <li>STRUCTURE, SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STRUCTURE SHALL MEET ALL SDIFFERENT THAN THOSE SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO ENGINEER IN WRITING.</li> <li>MINIMUM VANG PLATE THICKNESS = 1/2".</li> <li>WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE WOST SEVERE EFFECT.</li> <li>THE DOLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.</li> <li>MAXIMUM DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5%. OF THE POLE HEIGHT UNDER THE DESIGN LIMIT.</li> <li>MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO TALK SET OF OF THE MULTINA THE DESIGN LIMIT.</li> <li>MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO TALK SET OF OF STRUCTURE SHALL BE CALVANIZED STEEL.</li> <li>ALL STRUCTURES SHALL BE GALVANIZED STEEL.</li> <li>ALL STRUCTURE SHALL ABE QUANIZED STEEL.</li> <li>ALL STRUCTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARVS, POLES, BASE PLATES, ETC.</li> </ul>



OAD	CASES





LENGTH

(FT)

105

ANGLE 🛛

-38

STR #

63

=== + T \_ \_ \_ \_ \_ \_ \_ SECTION F-F (N.T.S.) TWO WAY ONE HOLE THROUGH VANG - VERTICAL

(N.T.S.)

LOADING TABLE								
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10
V1	700	500	1200	600	500	800	400	900
T1	-4100	-5900	-3200	-2100	-3100	-1900	-1100	-3200
L1	200	600	1100	4600	3000	4500	300	700
V2	2400	1500	2700	1900	1200	1800	900	2900
T2	-10700	-15400	-7700	-6000	-8500	-4500	-2000	-9700
L2	2900	3600	3300	13600	9600	11200	1300	1200
V3	900	700	1400	700	500	900	300	3500
Т3	-4600	-6300	-3700	-2500	-3500	-2200	-700	-3500
L3	1200	1600	1700	5700	4300	5400	400	1100
V4	500	400	1000	300	300	600	200	500
T4	-2800	-4800	-2300	-1500	-2600	-1400	-400	-1700
L4	600	700	1000	2800	2100	3000	100	300
V5	500	300	1000	300	300	600	200	300
T5	-1300	-1600	-1500	-800	-1000	-900	-300	-600
L5	300	600	700	1600	1900	2100	300	400
V6	600	500	1300	400	400	700	200	400
Т6	-1400	-2200	-1700	-800	-1300	-1000	-200	-500
L6	100	400	400	1500	2300	2300	100	200
V7	600	200	2300	700	200	2500	200	200
Τ7	-100	-200	100	-100	-200	100	100	-100
L7	200	300	400	200	300	400	100	100
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE. LOAD CASES
CASE 1 NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2 NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3 NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4 NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5 NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00

CASE 6 NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00

CASE 7 DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00

CASE 10 STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50

<u>WIRE DATĂ</u>

OHGW: "7#9" ALUMOWELD

115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC

12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR

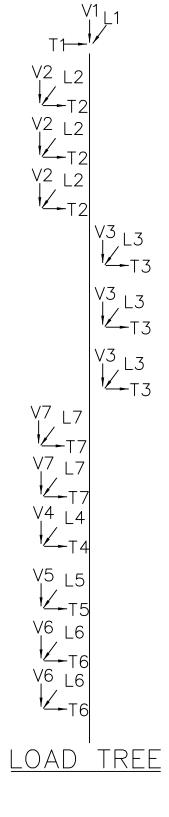
ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

NOTES:

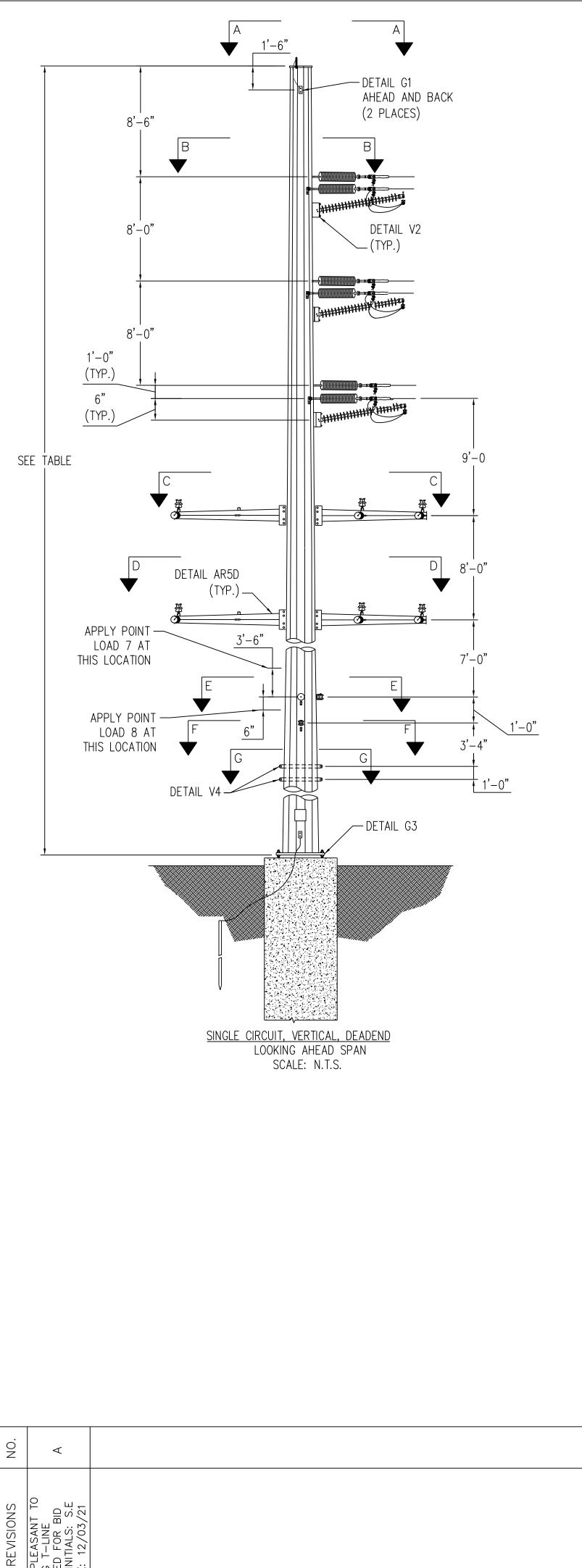
- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE OVERLOAD FACTORS AND INSULATOR WEIGHT.
- 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".
  7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN
- THE MOST SEVERE EFFECT.
  8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY
- BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES
- WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL.

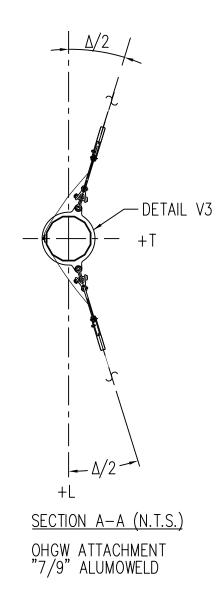
11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.

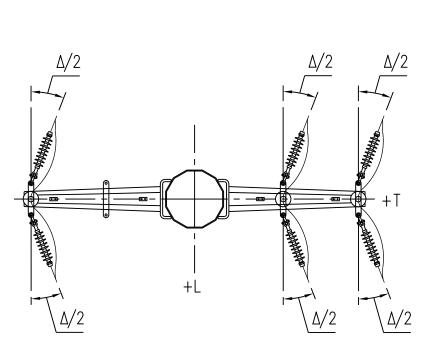
12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.



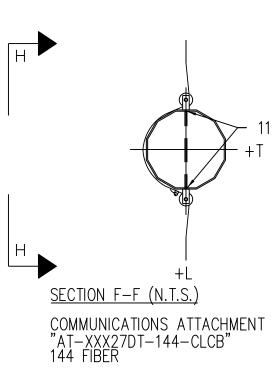
			Greenville Utilities	GREENVILLE U Greenville, North	
	FOR	RID		115kV TRANSMIS MT. PLEASANT SU LOAD AND E DEND 30°-60° WI	IB TO SUGG DESIGN
			DWN.D. CHAMBLI CKD. R. DILLABO SCALE: NONE	ISS DATE 12/03/21 OUGH APPD. S. ECKMAN	DWG. NO. DE-60L_STR-63





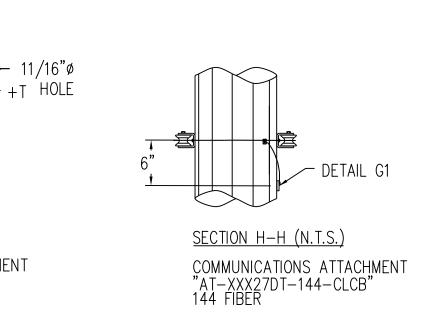


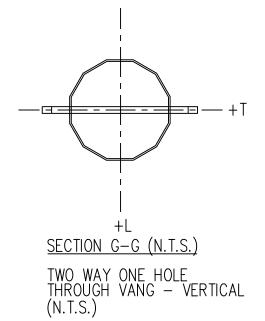
<u>SECTION C-C, D-D (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC



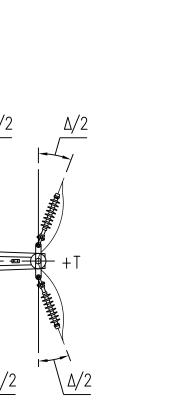
NO.	A	
REVISIONS	MT. PLEASANT TO SUGG T-LINE ISSUED FOR BID ENG.INITIALS: S.E DATE: 12/03/21	

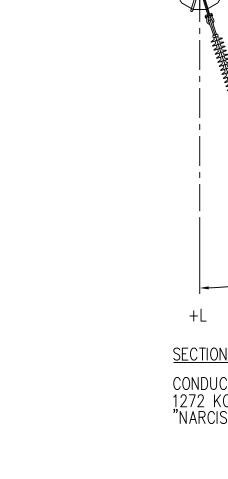


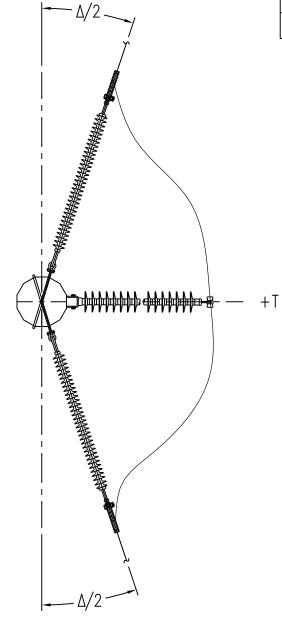




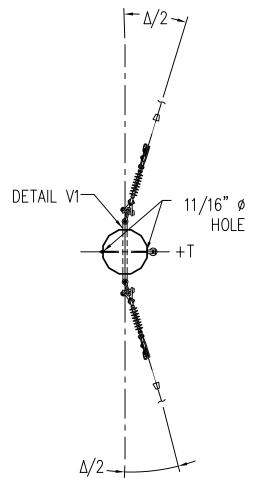
+L <u>SECTION E-E (N.T.S.)</u> NEUTRAL ATTACHMENT 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR







<u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC



LENGTH (FT)

80

ANGLE **A** 

38

STR #

6

LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 1
V1	-100	-200	200	-200	-200	300	-100	300
T1	3400	5300	2700	2000	2800	1400	800	2500
L1	1800	700	400	4600	2600	3400	1100	2500
V2	-300	-300	200	-500	-400	400	-100	800
T2	10300	13900	7100	5500	7400	4000	1900	9700
L2	1500	1700	1700	12300	7700	9600	700	700
V3	-300	-400	100	-300	-300	200	-100	3600
Т3	6300	7000	4700	3400	3700	2600	1200	5800
L3	1100	1200	1300	8400	5500	6800	500	1200
V4	-200	-300	200	-200	-200	200	-100	400
T4	4200	5300	3300	2200	2800	1800	700	3600
L4	300	600	700	4900	3300	4500	200	200
V5	-100	-100	400	-100	-100	300	-100	100
T5	1300	1400	1300	700	800	700	500	700
L5	300	400	400	1700	1700	1800	100	100
V6	-200	-100	500	-200	-100	400	-100	100
T6	1300	1900	1600	700	1000	900	300	500
L6	100	200	200	1500	2000	2200	100	100
V7	400	100	1100	400	100	1200	100	200
Т7	-1200	-600	-1000	-1200	-600	-1000	-400	-1300
L7	1900	1100	1700	1900	1100	1700	500	2000
V8	400	100	1100	400	100	1200	100	200
Т8	-1200	-600	-1000	-1200	-600	-1000	-400	-1300
L8	1900	1100	1700	1900	1100	1700	500	2000
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

> T1--'' V2 L2|  $V_{L2}^2$  $V^2_{L2}$ V<sup>3</sup> L3 V<sup>3</sup>L3 V<sup>3</sup>L3 ₩\_T3 K-T3 V3 L3  $\begin{array}{cccc} V_{3} & V_{3} & V_{3} \\ V_{-T3} & V_{-T3}^{3} \end{array}$ Ϋ7 L7 V4 L4 V8 L8 V8 L8 V5 L5 V6 L6 V6 L6 12-T6 LOAD TREE

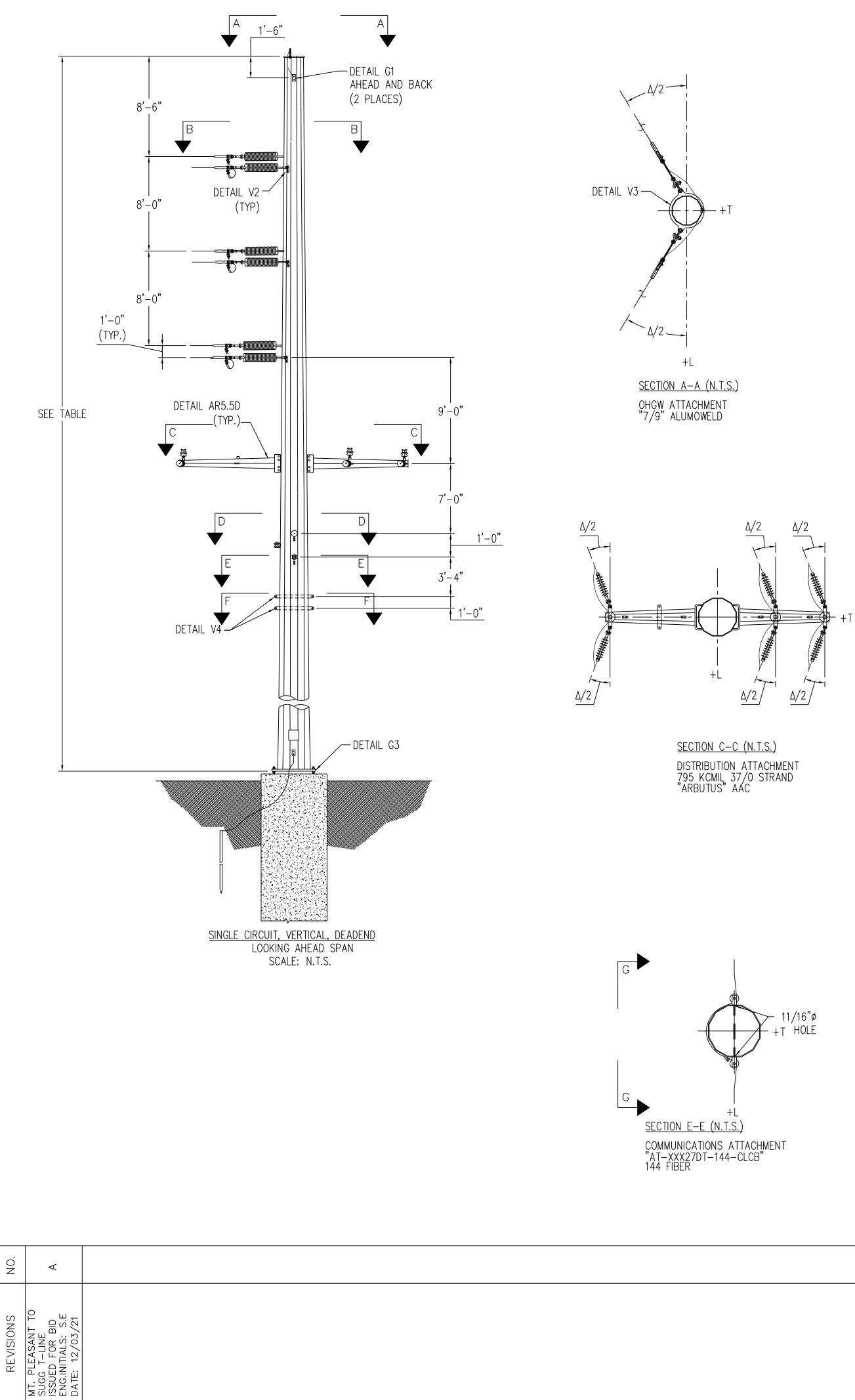
### LOAD CASES

CASE	1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50							
CASE	2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
CASE	3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
CASE	4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50							
CASE	5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
CASE	6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00							
CASE	7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00							
CASE	10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50							
WIRE	DAT	Ā							
		49" ALUMOWELD							
		72 KCMIL 61/0 STRAND "NARCISSUS" AAC							
		795 KCMIL 37/0 STRAND "ARBUTUS"AAC							
		ION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR —XXX27DT—144—CLCB" 144 FIBER							
AD22:	A I	-XXX2/DI-I44-CLCB I44 FIBER							
<u>NO I</u>	<u>ES:</u>								
1. AL	L ST	ATED LOADS ARE ULTIMATE VALUES AND INCLUDE							
0\	/ERLC	DAD FACTORS AND INSULATOR WEIGHT.							
		TURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE							
		ANEOUS APPLICATION OF DEAD LOAD, WIND ON THE							
		TURE, AND WIRE LOADS FOR EACH LOADING CASE. TURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE							
		ERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL							
REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.									
4. W]									
	SHAPE FACTOR OF 1.0.								
		CATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT							
		THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS BE SUBMITTED TO ENGINEER IN WRITING.							
		JM VANG PLATE THICKNESS = $1/2$ ".							
		SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN							
		OST SEVERE EFFECT.							

THE MUST SEVERE EFFECT. 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.

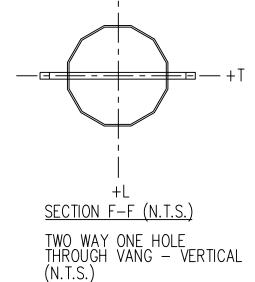
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.





- DETAIL G1 <u>SECTION G-G (N.T.S.)</u>

COMMUNICATIONS ATTACHMENT "AT-XXX27DT-144-CLCB" 144 FIBER



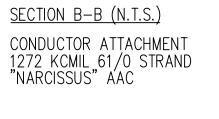
<u>SECTION D-D (N.T.S.)</u> NEUTRAL ATTACHMENT 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

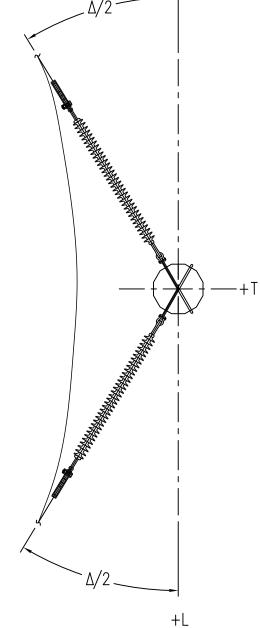
<sup>Δ</sup>/2

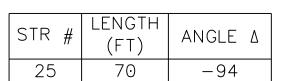
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— DETAIL V1

11/16"ø HOLE







LOADING TABLE									
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10	
V1	100	-100	300	-100	-100	200	100	100	
T1	-5200	-6500	-5100	-2600	-3300	-2700	-1000	-3700	
L1	-100	-100	-200	-2100	-1500	-2300	-100	-300	
V2	100	-300	400	-200	-300	300	100	600	
T2	-20700	-20000	-15200	-10800	-10500	-8100	-4400	-20900	
L2	-600	-700	-700	-9200	-6000	-7200	-200	-300	
V3	200	-300	300	-200	-200	200	100	3700	
Т3	-13600	-11400	-10600	-7000	-6000	-5600	-2800	-13000	
L3	-500	-500	-500	-6200	-4200	-5100	-200	-800	
V4	100	-200	300	-100	-200	200	100	400	
T4	-9000	-8400	-7400	-4700	-4400	-3900	-1800	-8400	
L4	-400	-300	-400	-4100	-2700	-3500	-100	-200	
V5	100	-100	500	-100	-100	300	100	100	
T5	-2600	-2700	-2800	-1400	-1400	-1500	-1000	-1600	
L5	-200	-200	-200	-1300	-1400	-1400	-100	-100	
V6	300	-100	600	-200	-100	400	100	200	
T6	-2400	-3400	-3400	-1300	-1800	-1700	-500	-900	
L6	-100	-100	-100	-1200	-1700	-1600	-100	-100	
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3	

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

	$\cap$
LOAD	CASES

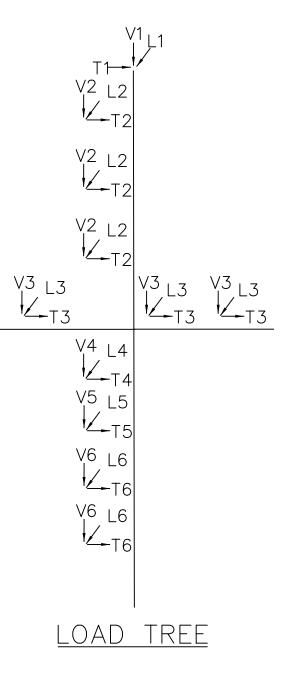
LUAD	<u>CASES</u>
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	) STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
	· - ·

<u>WIRE DATA</u>

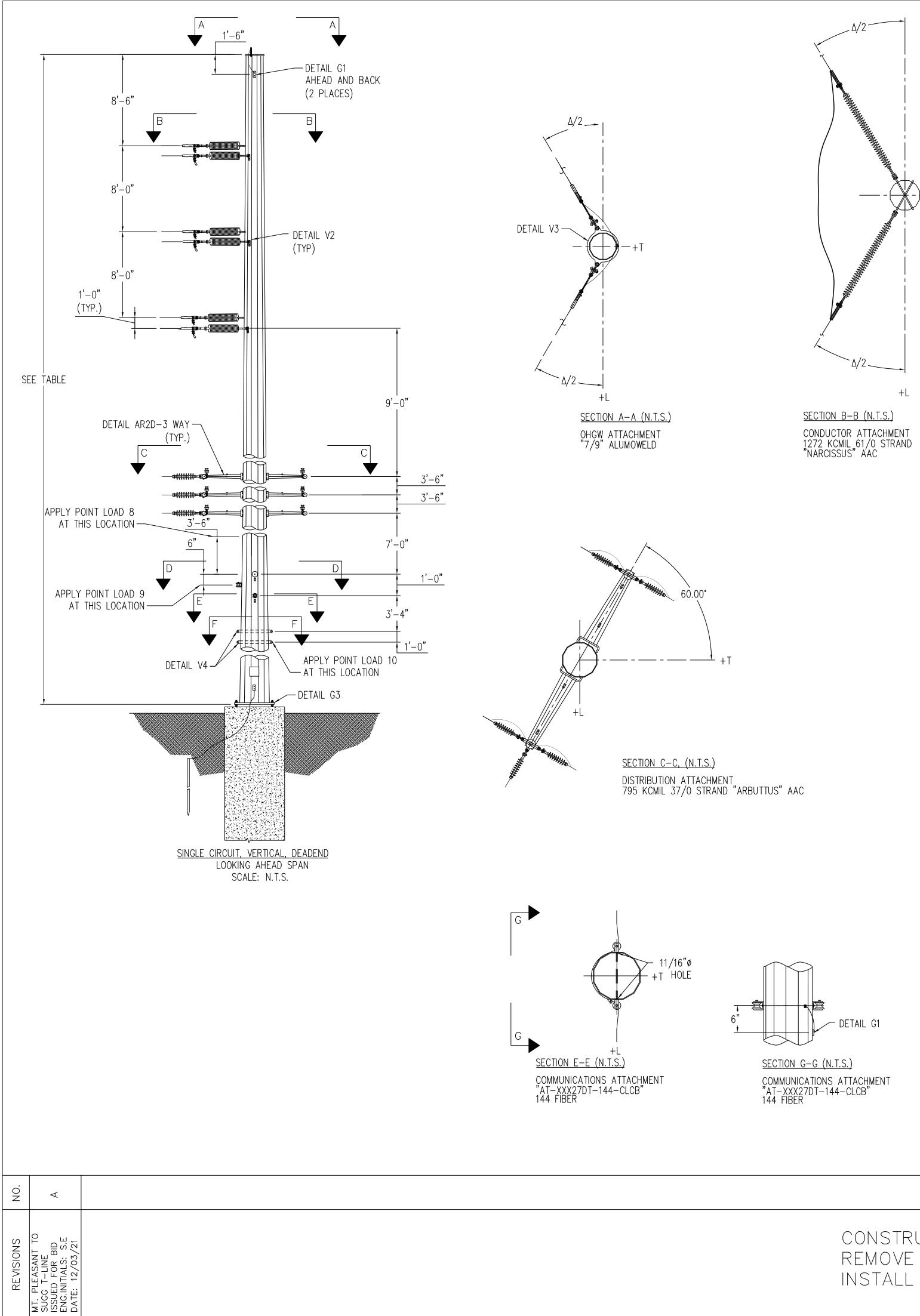
OHGW: "7#9" ALUMOWELD

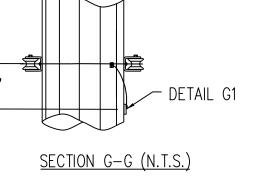
115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 795 KCMIL 37/0 STRAND "ARBUTUS"AAC DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER NOTES:

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL
- REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS. 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

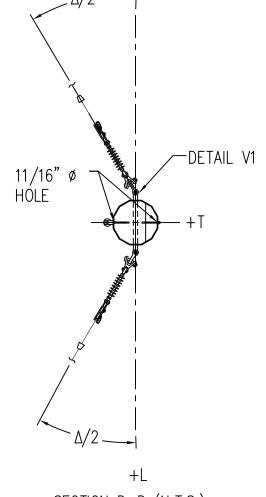


		Greenville Utilities	GREENVILLE Greenville, North		
ISSUED FOR	BID	115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN DEADEND 60°-90° WITH UNDERBUILD			
			ISS DATE 12/03/21 JGH APPD. S. ECKMAN	DWG. NO. DE-90L_STR-25	









LENGTH

(FT)

95

STR #

<u>52</u> I

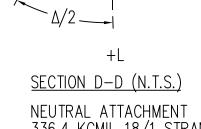
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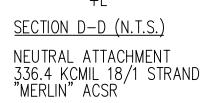
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Δ/2\_\_\_

+L

NGTH (FT) ANGLE Δ 95 -89	LOAD         CASE 1           V1         300           T1         -5000           L1         100           V2         1000           T2         -20500           L2         100           V3         400           T3         -6800           L3         6500           V4         -500           T4         -1000           L4         1600           V5         -500           T5         -1000           L5         1600           V6         200           T6         -2700           L6         100           V7         400           T7         -2400           L7         100           V8         500           T8         1300           L8         -1800           V9         2100           T9         4700           L9         900           V10         1400           T10         1200           L10         300           W(PSF)         10           ALL         LOADS           I	500 500 600 300 36.9 ARE IN OVERL	600 3200 1300 300 4.1 LBS, A .OAD F.	ACTORS	200 -6700 -100 600 -21100 -200 200 -6100 -4500 -4500 -4500 -2900 100 -2900 100 -2900 100 -2800 -100 100 -2800 -100 100 -3500 -100 100 -3500 -100 100 -3500 -100 100 -3500 -100 100 -3500 -100 100 -3500 -100 100 -3500 -100 -100 -3500 -100 -100 -3500 -100 -100 -3500 -100 -100 -100 -3500 -100 -100 -100 -100 -3500 -100 -100 -100 -100 -100 -100 -3500 -100 -100 -100 -100 -100 -100 -100 -	CASE 6A 700 -5100 100 1500 -15300 100 -5400 -5400 -200 -3700 3700 -200 -3700 3700 -200 -3700 3700 -2900 100 -3300 100 -3300 100 -2900 100 -3300 100 -2900 100 -3300 100 -3300 100 -3300 100 -3300 100 -3300 100 -3300 100 -3300 100 -3300 -3300 100 -3300 -300 -3300 -300	200 -2500 -2200 600 -10400 -9600 - - - - - - - - - - - - - - - - - -	CASE 5B -100 -3400 -1600 -300 -10700 -6300 -0 -10700 -4500 -2900 -100 -4500 -2900 -100 -100 -1500 -100 -150	CASE 6B 400 -2600 -2500 800 -7800 -7800 -7500 -3700 -300 -3	CASE 7         100         -1000         100         500         -4400         100         200         -1500         1500         -200         400         -200         400         -200         400         -200         400         -200         400         -200         400         -200         400         100         200         400         100         200         100         200         100         200         400         100         200         400         100         200         400         500         100         500         100         0	CASE 10         200         -3300         100         900         -20300         100         300         -20300         100         300         -20300         100         300         -20300         100         300         -20300         100         -3100         -1000         1300         -1000         1300         -1000         1300         -1000         1300         -1500         100         200         1500         -1900         1300         5800         1400         800         1000         300         3	CASE 1 CASE 2 CASE 3 CASE 4 CASE 4 CASE 4 CASE 5 CASE 5 CASE 5 CASE 6 CASE 6 CASE 6 CASE 6	CASES NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50 NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00 NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00 NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50 NESC MEDIUM MAIN ALIGNMENT DEADEND: 15°, .25" ICE, 4 PSF WIND, OLF: L=1.65, T=2.50, V=1.50 NESC MEDIUM ONE-WAY DEADEND: 15°, .25" ICE, 4 PSF WIND, OLF: L=1.65, T=2.50, V=1.50 NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00 NESC HIGH WIND MAIN ALIGNMENT DEADEND: 60°, 0" ICE, 120 MPH WIND, OLF: L=1.00, T=1.00, V=1.00 NESC HIGH WIND ONE-WAY DEADEND: 60°, 0" ICE, 120 MPH WIND, OLF: L=1.00, T=1.00, V=1.00 NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00 NESC ICE WITH WIND MAIN ALIGNMENT DEADEND: 15°, 1" ICE, 40 MPH WIND, OLF: L=1.00, T=1.00, V=1.00 NESC ICE WITH WIND MAIN ALIGNMENT DEADEND: 15°, 1" ICE, 40 MPH WIND, OLF: L=1.00, T=1.00, V=1.00 NESC ICE WITH WIND ONE-WAY DEADEND: 15°, 1" ICE, 40 MPH WIND, OLF: L=1.00, T=1.00, V=1.00 STEINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
DETAIL V1							V3	V2 V2 V2	-T2 L2 -T2 L2 -T2	V4 L4		115KV: 1 12.47kV: DISTRIBU ADSS: "A <u>NOTES</u> 1. ALL S OVER 2. STRU SIMU STRU 3. STRU NOT REQU 4. WIND SHAP	7#9" ALUMOWELD 272 KCMIL 61/0 STRAND "NARCISSUS" AAC 795 KCMIL 37/0 STRAND "ARBUTUS"AAC JTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR AT—XXX27DT—144—CLCB" 144 FIBER
L <u>D (N.T.S.)</u> TACHMENT 18/1 STRAND SR	ı— +T						V3  /       	V5 V9 V6 V7 V7 V7	-T4 -T4 -T4 -T4 -T4 -T4 -T8 -T5 L9 -T5 L9 -T9 L6 -T6 L7 V7 -T7 L7 V10 1/	V4 L4 V4 L4 V4 L4 V4 L4 V4 L4 V4 L4 V4 L7 T4	- - -	SHAL 6. MININ 7. WIND THE 8. THE OF T BE C 9. MAXI LIMIT WITH 10. ALL S 11. ORIEN DRAW 12. MANU CREA	THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS L BE SUBMITTED TO ENGINEER IN WRITING. MUM VANG PLATE THICKNESS = 1/2". SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN MOST SEVERE EFFECT. DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% HE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY AMBERED TO FALL WITHIN THE DESIGN LIMIT. MUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE 'ED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. STRUCTURES SHALL BE GALVANIZED STEEL. NT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON /INGS. JFACTURER SHALL APPLY POINT LOADS NECESSARY TO TE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING 5, POLES, BASE PLATES, ETC.
+L <u>SECTION F-F (N.T.S.)</u> TWO WAY ONE HOLE THROUGH VANG – VE (N.T.S.)	RTICAL							<u>Loa</u>	 <u>D T</u> F	<u>REE</u>			





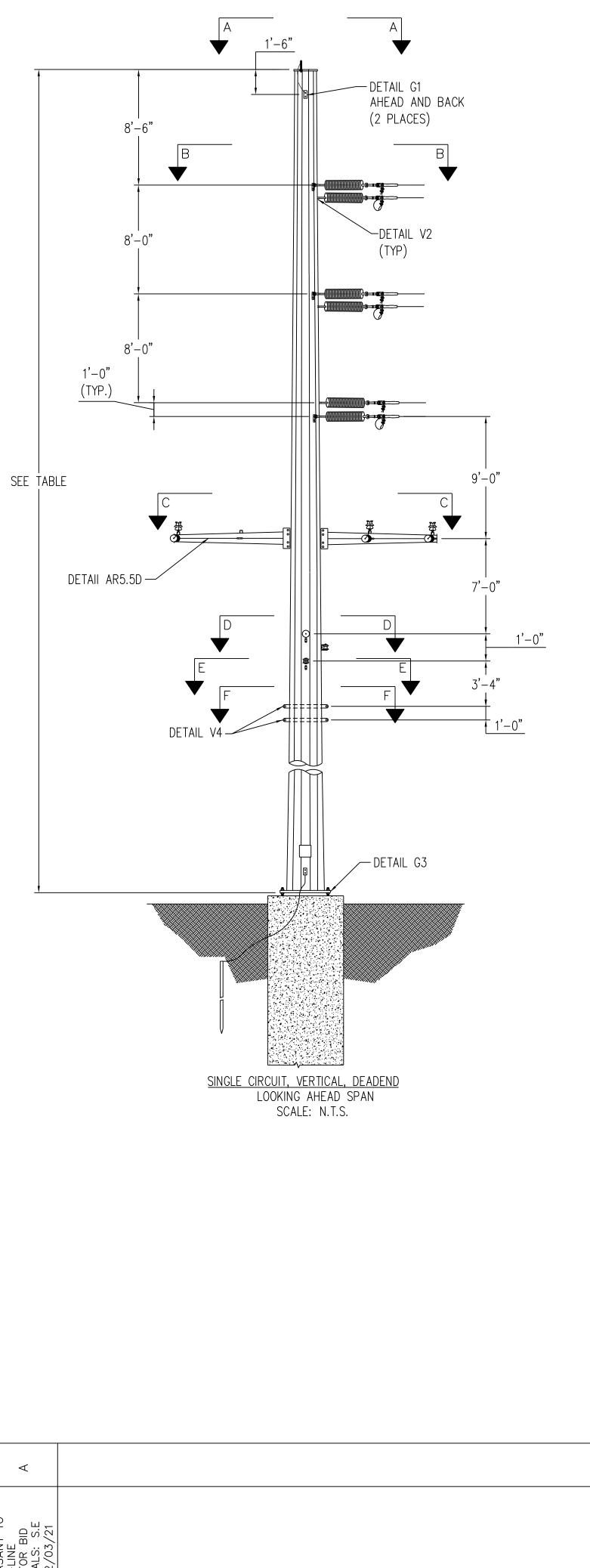


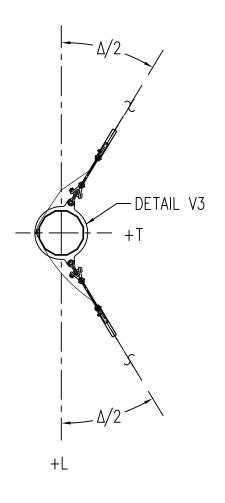
GREENVILLE UTILITIES Greenville, North Carolina

115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN DEADEND 60°-90° WITH UNDERBUILD

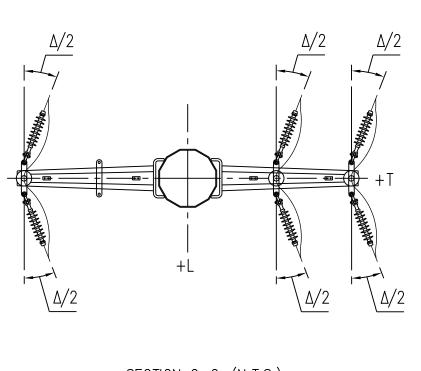
ISSUED FOR BID

DWN.D. CHAMBLISS DATE 12/03/21CKD. R. DILLABOUGH APPD. S. ECKMAN SCALE: NONE

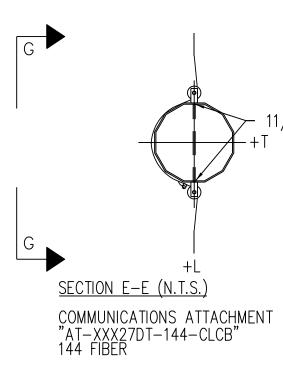




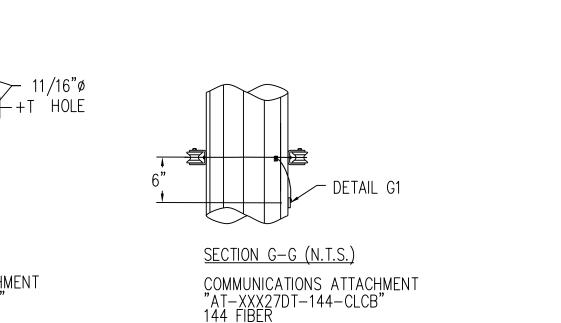
<u>SECTION A-A (N.T.S.)</u> OHGW ATTACHMENT "7/9" ALUMOWELD

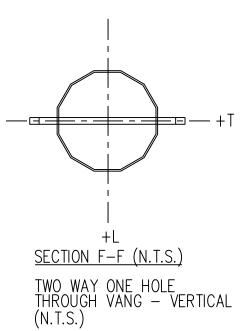


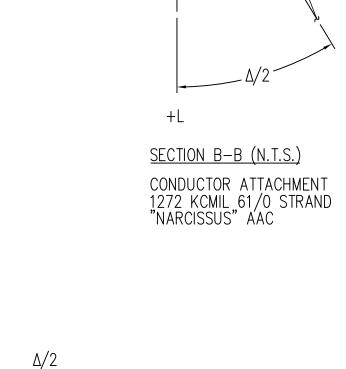
<u>SECTION C-C, (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC



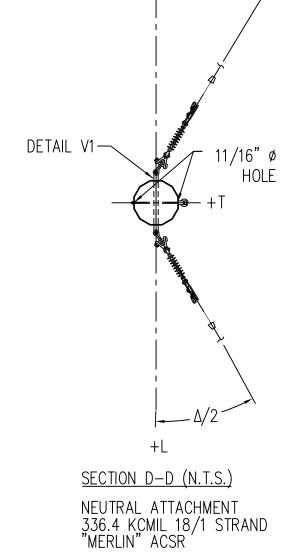
NO.	A	
REVISIONS	MT. PLEASANT TO SUGG T-LINE ISSUED FOR BID ENG.INITIALS: S.E DATE: 12/03/21	







— +T



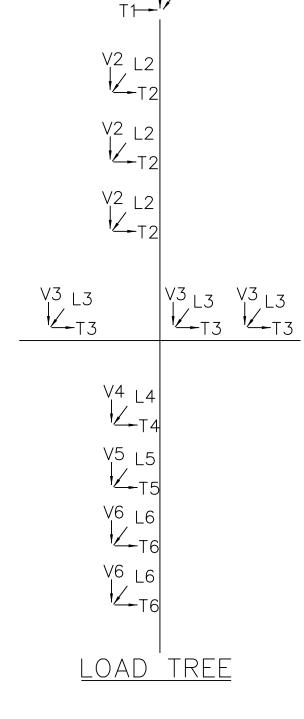
LOADING TABLE									
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10	
V1	300	200	600	300	200	500	100	200	
T1	6300	7100	5200	3800	3900	2900	1800	5300	
L1	1400	700	-600	-3500	-2100	-2700	700	1600	
V2	1100	600	1300	1000	600	1100	500	1100	
Т2	19300	19500	13900	9900	9900	7200	4000	20000	
L2	100	200	-200	-8700	-5400	-6600	100	200	
V3	700	400	1000	600	400	800	300	4700	
Т3	12400	10800	9600	6300	5500	4900	2500	12200	
L3	200	200	-300	-6100	-3900	-4800	100	900	
V4	600	400	900	400	300	600	200	600	
T4	8300	8100	6600	4200	4200	3400	1600	7900	
L4	200	100	-200	-3900	-2500	-3300	100	100	
V5	300	200	700	200	200	400	100	200	
T5	2400	2400	2500	1200	1300	1300	900	1500	
L5	100	100	-100	-1300	-1300	-1300	100	100	
V6	400	300	900	300	200	500	200	200	
Т6	2400	3300	3100	1200	1700	1600	500	900	
L6	100	100	-100	-1200	-1700	-1600	100	100	
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3	

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

_Δ/2			
	STR #	LENGTH (FT)	ANGLE A
	23	95	89

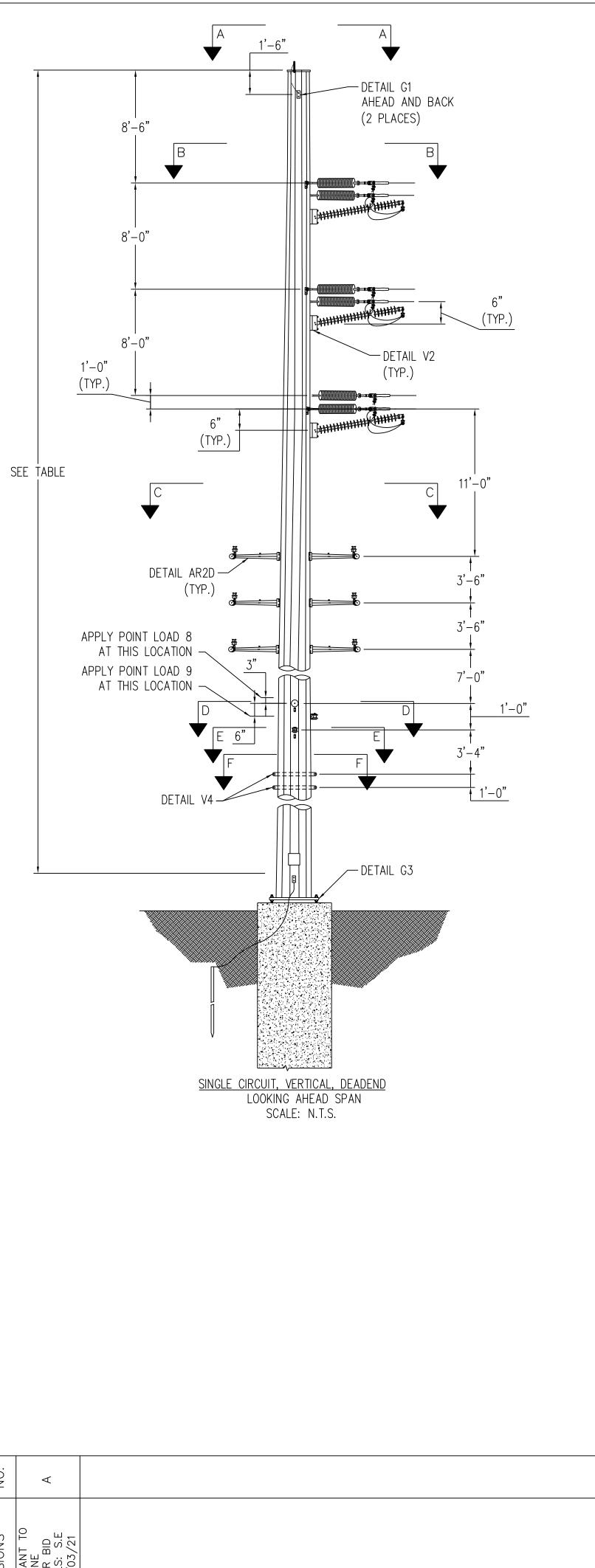
LOAD (	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
WIRE DA	ТА
	#9"ALUMOWELD
	72 KCMIL 61/0 STRAND "NARCISSUS" AAC
	795 KCMIL 37/0 STRAND "ARBUTUS"AAC
	TION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR
	T-XXX27DT-144-CLCB"144 FIBER
<u>NOTES:</u>	
	TATED LOADS ARE ULTIMATE VALUES AND INCLUDE
	OAD FACTORS AND INSULATOR WEIGHT. TURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE
2. 01100	TORE AND ATTACONNERTS SHALE DE DESIGNED FOR THE

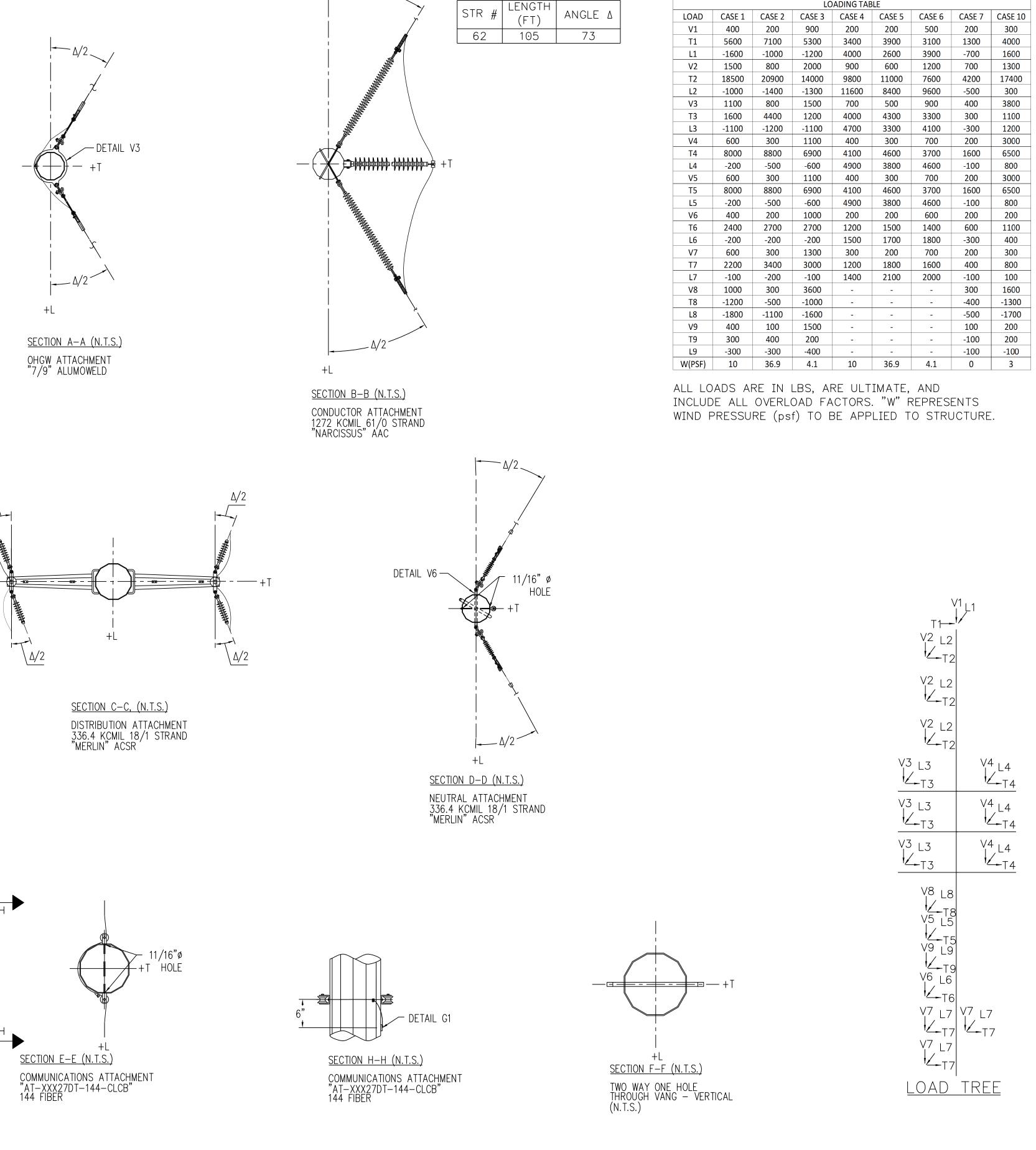
- SIRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT. 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5%
- OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

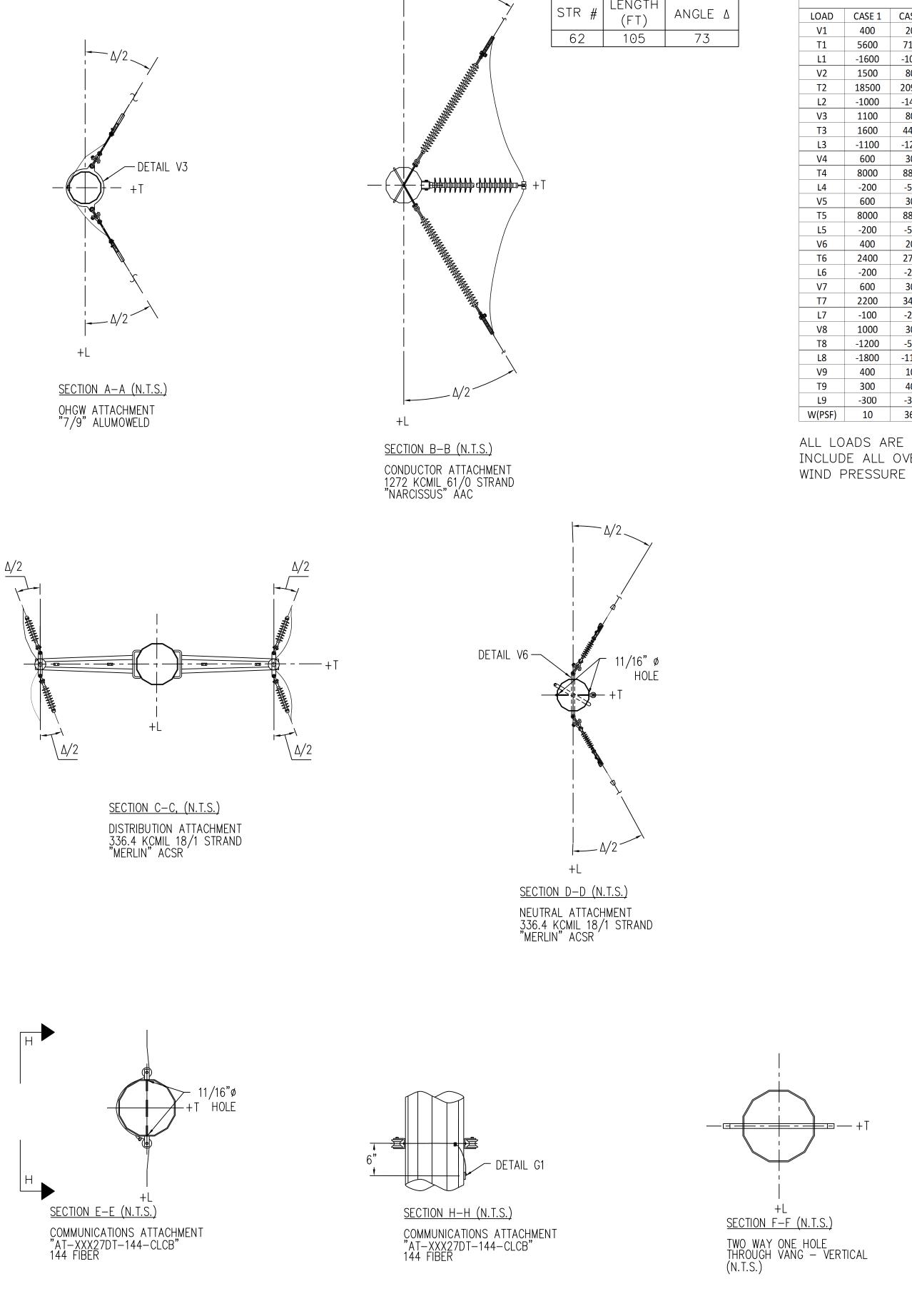


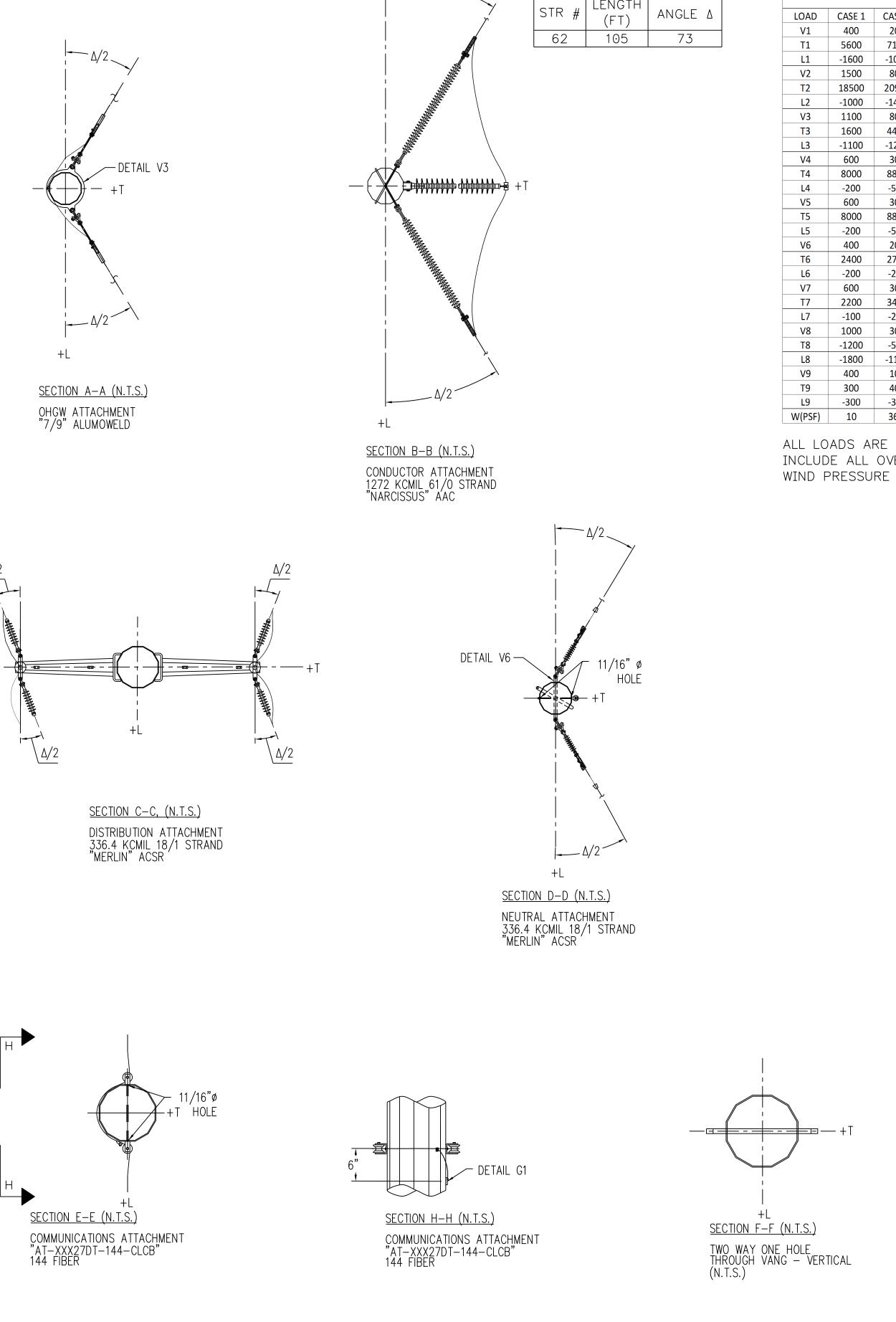
V<sup>1</sup> L1

		Greenville Utilities	GREENVILLE Greenville, Nor		
ISSUED FOR	חום סר	115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN DEADEND 60°-90° WITH UNDERBUILD			
ISSUED FU	JR DIU	DWN.D. CHAMBLISS	DATE 12/03/21		
		CKD. R. DILLABOUGH SCALE: NONE	, ,	DWG. NO. DE-90R_STR-23	









NO.	A	
REVISIONS	MT. PLEASANT TO SUGG T-LINE ISSUED FOR BID ENG.INITIALS: S.E DATE: 12/03/21	



LC	LOADING TABLE							
3	CASE 4	CASE 5	CASE 6	CASE 7	CASE 10			
)	200	200	500	200	300			
0	3400	3900	3100	1300	4000			
0	4000	2600	3900	-700	1600			
0	900	600	1200	700	1300			
0	9800	11000	7600	4200	17400			
0	11600	8400	9600	-500	300			
0	700	500	900	400	3800			
0	4000	4300	3300	300	1100			
0	4700	3300	4100	-300	1200			
0	400	300	700	200	3000			
0	4100	4600	3700	1600	6500			
)	4900	3800	4600	-100	800			
0	400	300	700	200	3000			
0	4100	4600	3700	1600	6500			
)	4900	3800	4600	-100	800			
0	200	200	600	200	200			
0	1200	1500	1400	600	1100			
)	1500	1700	1800	-300	400			
0	300	200	700	200	300			
0	1200	1800	1600	400	800			
)	1400	2100	2000	-100	100			
0	-	-	-	300	1600			
0	-	-	-	-400	-1300			
0	-	-	-	-500	-1700			
0	-	-	-	100	200			
	-	-	-	-100	200			
)	-	-	-	-100	-100			
	10	36.9	4.1	0	3			

-1200

-1100

-600

-600

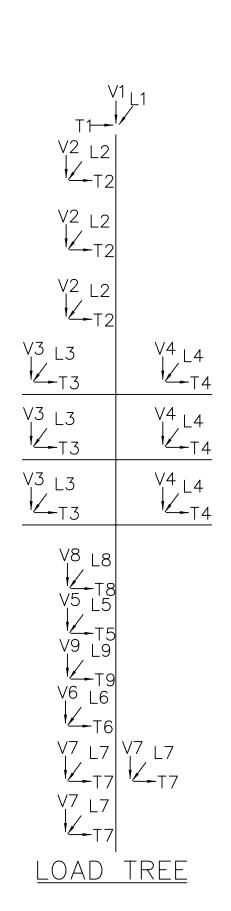
-200

-100

-1000

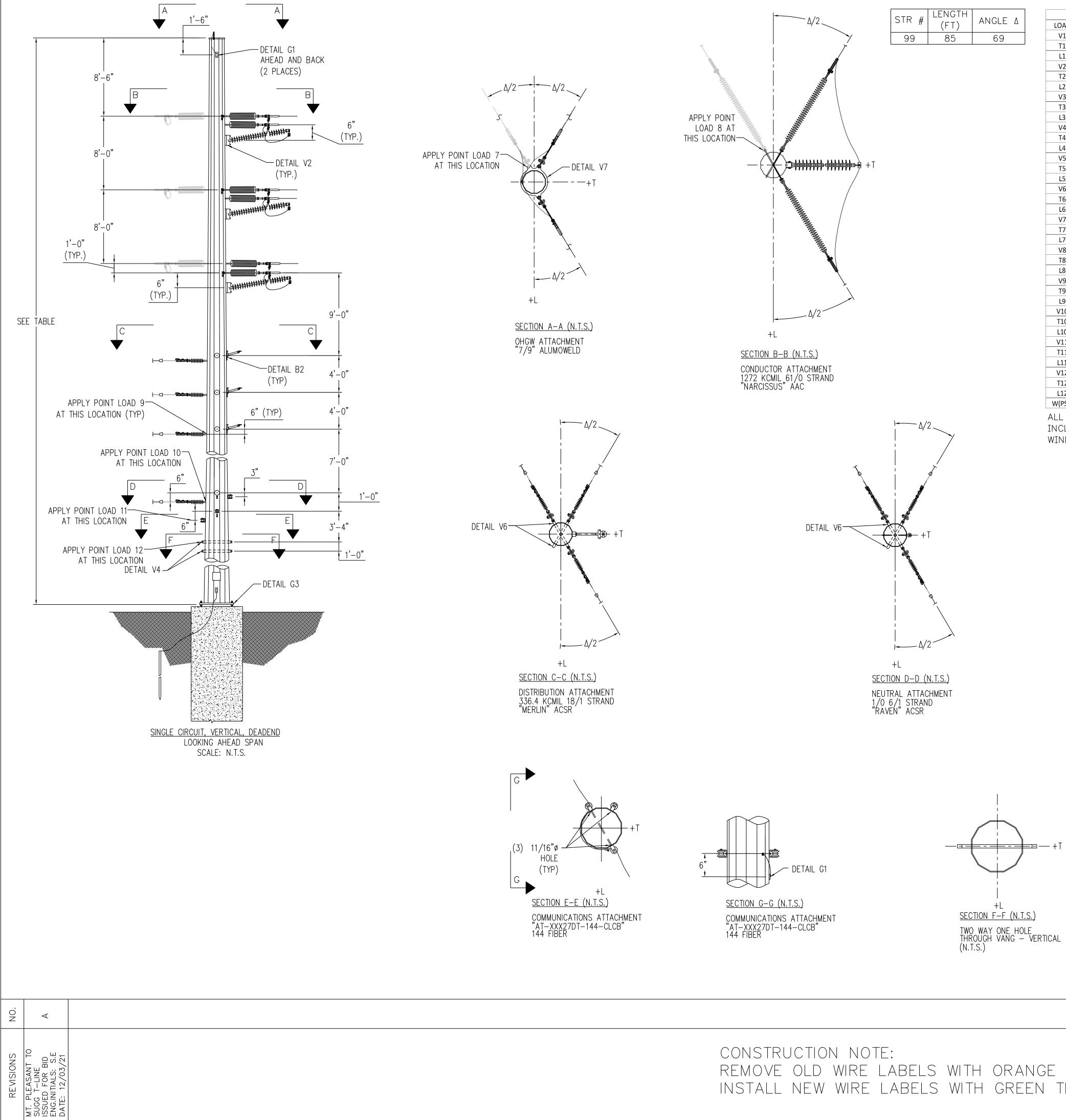
-1600

-1300 11600



<u>load ca</u>	ASES
	ESC MEDIUM: 15°, .25" ICE, 4 PSF WIND DLF: L=1.65, T=2.50, V=1.50
	ESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND DLF: L=1.00, T=1.00, V=1.00
	ESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND LF: L=1.00, T=1.00, V=1.00
	ESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND LF: L=1.65, T=2.50, V=1.50
	ESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND LF: L=1.00, T=1.00, V=1.00
	ESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND LF: L=1.00, T=1.00, V=1.00
	EFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL DLF: L=1.00, T=1.00, V=1.00
	TRINGING: -20°, 0" ICE, 2 PSF WIND DLF: L=1.50, T=1.50, V=1.50
ADSS: "AT- <u>NOTES</u> : 1. ALL STA OVERLOA 2. STRUCTU 3. STRUCTU 3. STRUCTU 3. STRUCTU 4. WIND PR REQUIRE 4. WIND PR SHAPE F 5. FABRICA THAN TH SHALL BI 6. MINIMUM 7. WIND SH THE MOS 8. THE DEFI OF THE BE CAME 9. MAXIMUN LIMITED WITH TH 10. ALL STRU 11. ORIENT S DRAWING	DN NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR XXX27DT-144-CLCB" 144 FIBER TED LOADS ARE ULTIMATE VALUES AND INCLUDE AD FACTORS AND INSULATOR WEIGHT. JRE AND ATTACHMENTS SHALL BE DESIGNED FOR THE NEOUS APPLICATION OF DEAD LOAD, WIND ON THE JRE, AND WIRE LOADS FOR EACH LOADING CASE. JRE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE RMITTED. STRUCTURE SHALL MEET ALL TECHNICAL MENTS OF THE STEEL POLE SPECIFICATIONS. RESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A "ACTOR OF 1.0. TOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT HOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS & SUBMITTED TO ENGINEER IN WRITING. M VANG PLATE THICKNESS = 1/2". HALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN ST SEVERE EFFECT. LECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BERED TO FALL WITHIN THE DESIGN LIMIT. M DEFLECTION AT TOP OF THE STRUCTURE SHALL BE TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES HE EXCEPTION TO THE 60°F NO WIND LOAD CASE. UCTURES SHALL BE GALVANIZED STEEL. SINGLE SIDED VANGS FOR HARDWARE SHOWN ON SS. CTURER SHALL APPLY POINT LOADS NECESSARY TO



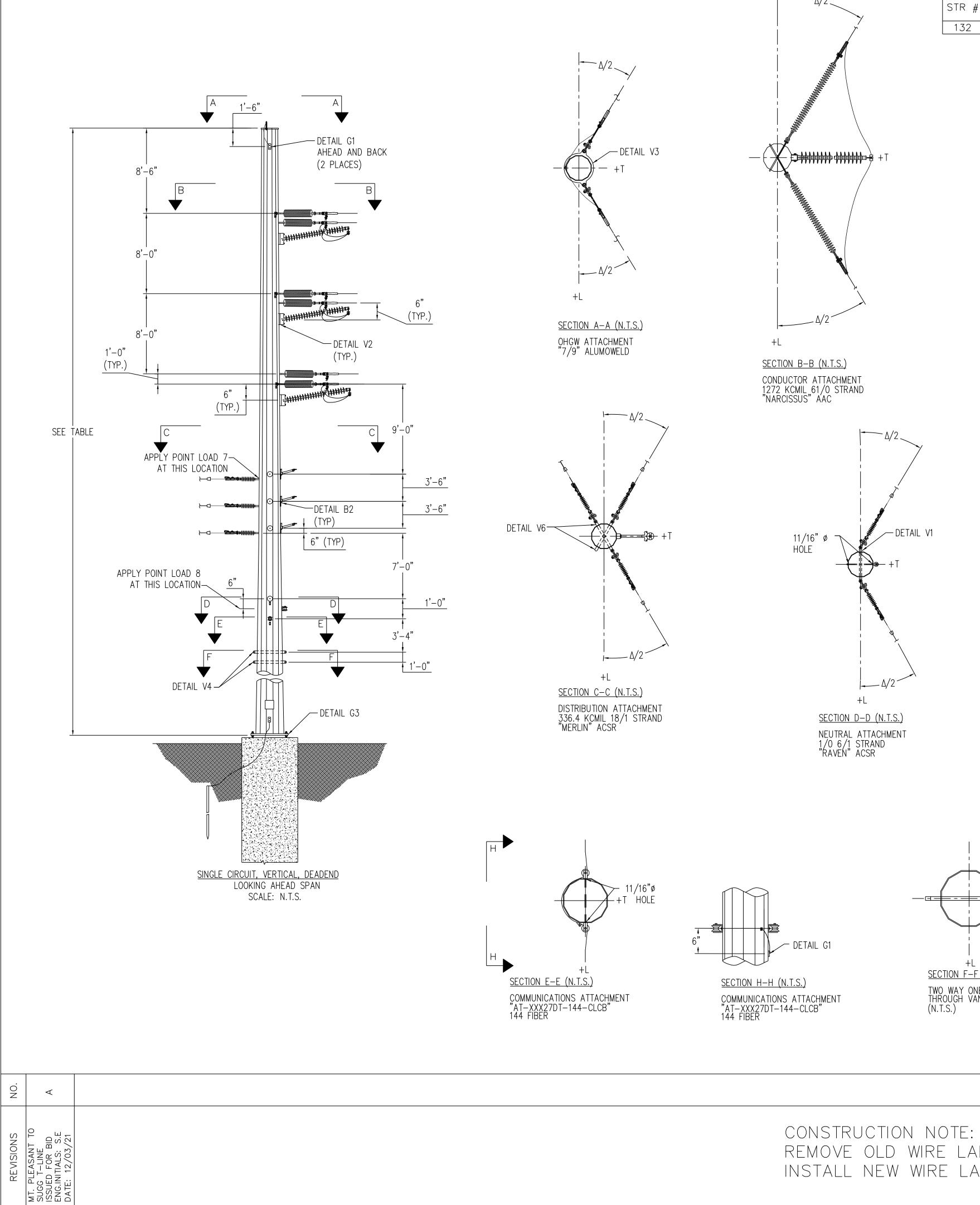


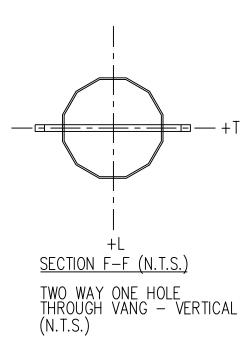
REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT

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112       2700       -1400       -2700       -1300       2500         112       2800       2500       2700       -1400       2700       1100       2500         112       2800       2500       2700       -1400       2700       1100       2500         112       2800       2500       2700       -1400       2500       2800       2800         112       2800       2500       2700       -1400       2500       2800       2800         110       369       41       0       369       41       0       3         ALL LOADS ARE IN LBS, ARE ULTIMATE, AND       369       41       0       3       41       0       3         NDT PERSURE (pst) TO BE APPLIED TO STRUCTURE.       "WIND PRESSURE (pst) TO BE APPLIED TO STRUCTURE.       "WIND PRESSURE (pst) TO BE APPLIED TO STRUCTURE.       SHAPE FACIOR OF 1.0.       SHAPE FACIOR OF 1.0.         110       MINDUM VANG PLATE THICKNESS = 1/2".       "WIND PRESSURE (pst) TO BE APPLIED TO STRUCTURE.       SHAPE FACIOR OF 1.0.       SHAPE FACIOR OF 1.0.         110       MINDUM VANG PLATE THICKNESS = 1/2".       "WIND SHALL BE SUBMITTED TO ENGINEES = 1/2".       SHAPE FACIOR OF 1.0.       SHAPE FACIOR OF 1.0.         110       MINIMUM VANG PLATE THICKNESS = 1/2".       "	CASE.
WIPSE       10       36.9       4.1       0       3         ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.       4. WIND PRESSURES SHOWN ON LOAD WORKSHET ARE BA SHAPE FACTOR OF 1.6.         SHAPE FACTOR OF 1.6.       5. FABRICATION MAY PROPOSE STRUCTURAL DETAILS DIFF THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIAT SHALL BE SUBMITTED TO ENGINEER IN WRITING.         WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.       5. FABRICATION MAY PROPOSE STRUCTURAL DETAILS DIFF THAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIAT SHALL BE SUBMITTED TO FORGINEER IN WRITING.         WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.       5. MINIMUM VANC PLATE THICKNESS = 1/2".         WIND SHALL BE APPLIED TO TO STRUCTURE.       5. MINIMUM VANC PLATE THICKNESS = 1/2".         WIND SHALL BE APPLIED TO TO STRUCTURE.       6. MINIMUM VANC PLATE THICKNESS = 1/2".         WIND SHALL BE CALL BE CALL BE CALL BE CALL BE LIMITED OF THE POLE HEIGHT UNDER THE DESIGN UMIT.       6. MINIMUM VANC PLATE THE MOST SEVERE EFFECT.         WIND TO THE TO 10% OF STRUCTURE HEIGHT UNDER ALL LO WITH THE EXCEPTION TO THE 60% NO WIND LOAD CASE. PI BE CAMBERED TO FAIL WITHIN THE DESIGN UMARE SHOWN O DRAWINGS.         V71_7       V1_11       T7       V1_11       9. L8       V2 L2       V3 L3       V2 L2       V2 L2       V2 L2       V2 L2       V2 L2       V2 L2       V	NICAL
<ul> <li>INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.</li> <li>5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFETHAN THOSE SHOWN TO SIMPLIFY FABRICATION.VARIAT SHALL BE SUBMITTED TO ENGINEER IN WRITING.</li> <li>6. MINIMUM VANG PLATE THICKNESS = 1/2".</li> <li>WIND SHALL BE SUBMITTED TO TO STRUCTURE SHALL BE SUBMITTED TO POSE PROCEED IN WHICH REST THE MOST SEVERE EFFECT.</li> <li>8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED OF THE POLE HEIGHT UNDER TALL ONDER ALL ONE AND CASE. PLATES SEVERE EFFECT.</li> <li>8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED OF THE POLE HEIGHT UNDER THE UNDER ALL ONE AND CASE. PLATES STALL BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.</li> <li>9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE CAUVANIZED STEEL.</li> <li>10. ALL STRUCTURES SHALL BE CAUVANIZED STEEL.</li> <li>11. ORIENT SINCLE SIDED VANCS FOR HARDWARE SHOWN OD DRAWINGS.</li> <li>12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY OR AND STRUCTURE SHALL APPLY POINT LOADS NECESSARY OREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS STARMS, POLES, BASE PLATES, ETC.</li> <li>V8 L8 V2 L2 V2 V2 V2 V2 V2 V2 V3 V3</li></ul>	
$-+T$ $V_{-T12}^{U-T3}$ $V_{-T3}^{U-T3}$ $V_{-T3}^{U-T3}$ $V_{-T3}^{U-T3}$ $V_{-T4}^{U-T4}$ $V_{-T11}^{U-T4}$ $V_{-T11}^{U-T4}$ $V_{-T11}^{U-T5}$ $V_{12L12}^{U-12}$ $V_{6}^{U-12}$ $V_{-T12}^{U-12}$ $V_{6}^{U-12}$ $V_{-T12}^{U-12}$ $V_{6}^{U-12}$ $V_{-T12}^{U-12}$	TED TO 1.5% C. POLES MAY SHALL BE LOAD CASES CASE. N ON ARY TO



Greenville Utilities GREENVILLE UTILITIES Greenville, North Carolina 115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN DEADEND 60°-90° WITH UNDERBUILD ISSUED FOR BID DWN.D. CHAMBLISSDATE 12/03/21CKD. R. DILLABOUGHAPPD. S. ECKMAN DWG.NO. DE-90R\_STR-99 SCALE: NONE





LOADING TABLE								
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CASE
V1	-100	-100	200	-100	-100	100	-100	100
T1	3700	5600	3600	1900	2800	1800	700	250
L1	100	100	100	2200	1500	2500	-100	100
V2	-500	-500	200	-400	-300	200	-100	900
Т2	14200	16200	10400	7600	8300	5500	2900	1420
L2	1700	800	900	10500	6400	8000	-400	170
V3	-200	-300	100	-200	-200	100	-100	210
Т3	6600	6900	5300	3400	3500	2700	1200	5800
L3	200	100	100	4500	2900	3800	-100	600
V4	-100	-200	100	-100	-100	100	-100	200
Τ4	3800	5000	3300	1900	2500	1700	800	290
L4	100	100	100	2200	1500	2100	-100	100
V5	-100	-100	300	-100	-100	200	-100	100
T5	2000	2000	2000	1000	1000	1100	800	1200
L5	100	100	100	1400	1400	1500	-	100
V6	-200	-200	300	-100	-100	200	-100	100
Т6	1800	2500	2500	1000	1300	1300	400	700
L6	100	100	100	1400	1800	1900	-100	100
V7	500	200	1200	-	-	-	200	300
Τ7	-1700	-700	-1400	-	-	-	-400	-220
L7	-4200	-2400	-3300	-	-	-	-900	-500
V8	300	100	900	-	-	-	100	200
Т8	-800	-400	-700	-	-	-	-300	-100
L8	-2100	-1300	-1900	-	-	-	-600	-220
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

LENGTH

(FT)

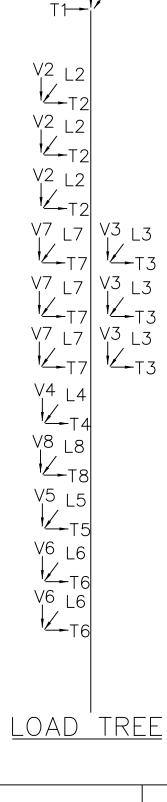
132 75

ANGLE **A** 

68

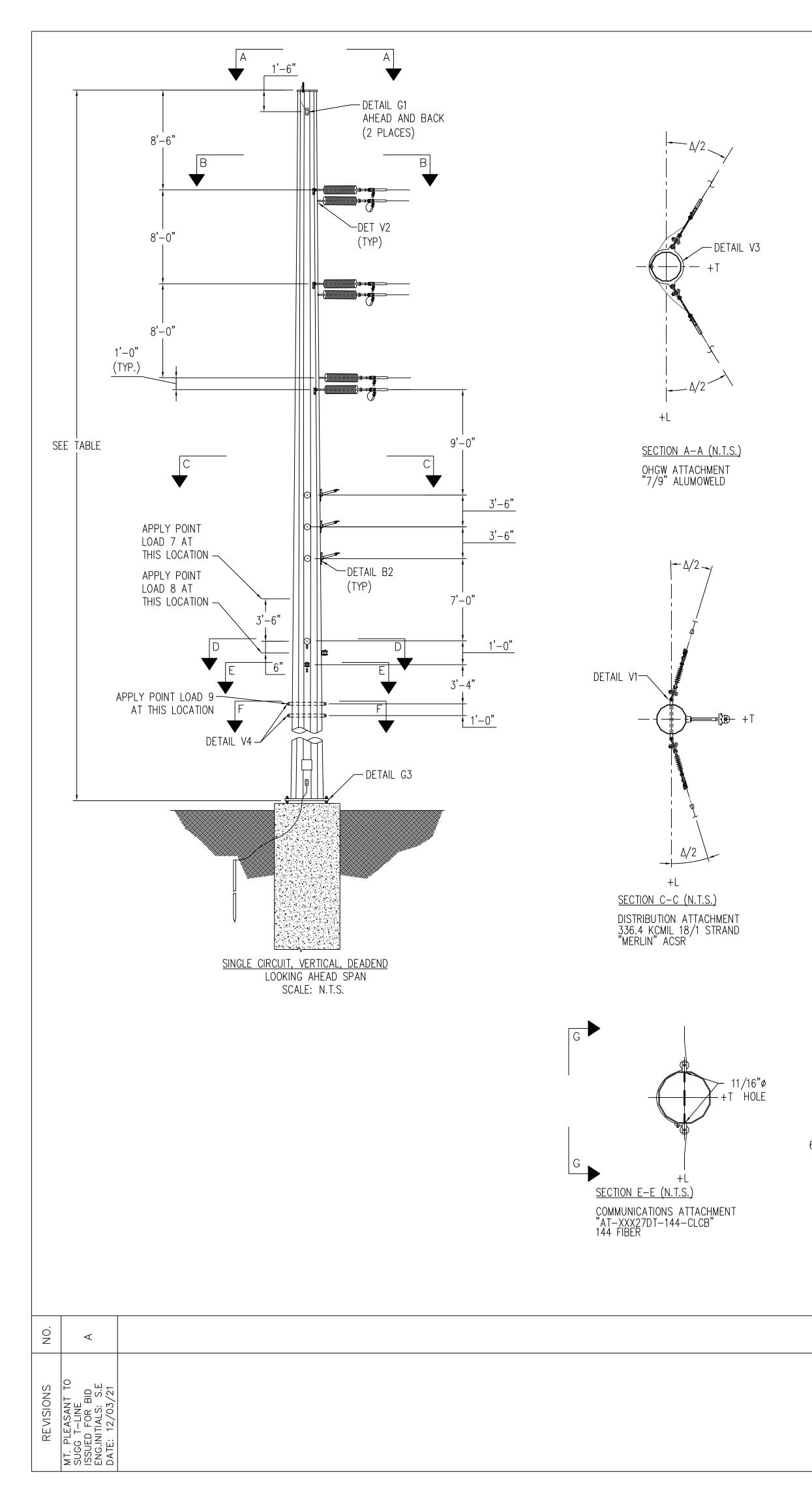
LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 4	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
WIRE DA	
	7#9"ALUMOWELD
	272 KCMIL 61/0 STRAND "NARCISSUS" AAC
	336.4 KCMIL 18/1 STRAND "MERLIN" ACSR JTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR
	AT-XXX27DT-144-CLCB" 144 FIBER
NOTES	
1. ALL S	- STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
OVER	LOAD FACTORS AND INSULATOR WEIGHT.
	CTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE LTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE
STRU	CTURE, AND WIRE LOADS FOR EACH LOADING CASE.
	CTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL
REQU	IREMENTS OF THE STEEL POLE SPECIFICATIONS.
	PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A
	ICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT
	THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS

- SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL.
- 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

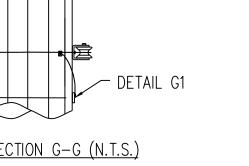


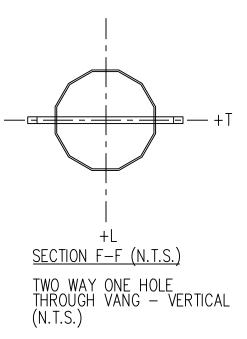
V1 L1

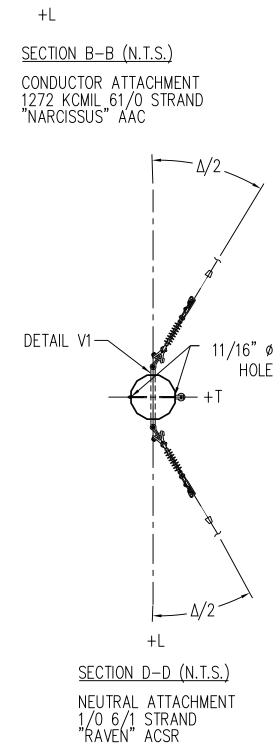
		Greenville Utilities	GREENVILLE ( Greenville, North	
ISSUED FOR	BID	MT	15kV TRANSMI PLEASANT SU LOAD AND ND 60°-90° W	UB TO SUGG
		DWN.D. CHAMBLISS CKD. R. DILLABOUGH SCALE: NONE	DATE 12/03/21 APPD. S. ECKMAN	DWG. NO. DE-90R_STR-132











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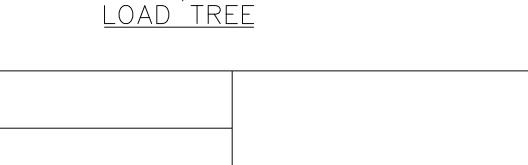
LOAD	CASE 1	CASE 2	CASE 3	CASE 4	CASE 5	CASE 6	CASE 7	CAS
V1	100	100	400	100	100	200	100	10
T1	6100	6900	5100	3900	4000	3200	1700	53
L1	-1600	-1100	-1300	-3500	-2200	-3000	-700	-14
V2	400	100	700	300	200	400	200	10
T2	19000	19000	13600	10500	10700	7900	3900	195
L2	-1400	-2000	-1700	-9400	-6300	-7400	-700	-50
V3	200	100	400	100	100	300	100	26
Т3	8300	8000	6600	4600	4600	3900	1500	75
L3	-900	-1000	-1100	-4200	-2900	-3700	-400	-80
V4	100	100	400	100	100	200	100	10
T4	4500	5500	3900	2500	3000	2300	900	350
L4	-400	-500	-600	-2100	-1500	-2100	-100	-10
V5	200	100	500	100	100	400	100	10
T5	2400	2400	2500	1400	1500	1500	1000	150
L5	-400	-500	-500	-1400	-1500	-1500	-	-10
V6	200	200	500	200	200	500	100	20
Т6	1200	1500	1400	1200	1500	1400	300	50
L6	-1200	-1500	-1500	-1200	-1500	-1500	-300	-60
V7	-	-	-	500	200	1800	200	30
T7	-	-	-	1400	700	1300	500	170
L7	-	-	-	-1600	-1000	-1400	-500	170
V8	-	-	-	500	200	1800	200	30
T8	-	-	-	1400	700	1300	500	170
L8	-	-	-	-1600	-1000	-1400	500	170
V9	-	-	-	1300	500	3000	400	70
Т9	-	-	-	-700	500	-1100	300	30
L9	-	-	-	1100	1300	1400	300	60
W(PSF)	10	36.9	4.1	10	36.9	4.1	0	3

INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

STR #	LENGTH (FT)	ANGLE <b>A</b>
153	85	91

# LOAD CASES

	0.05.40	LOA	AD (	CASES
7 )	CASE 10 100	CASE	E 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND
0	5300			OLF: L=1.65, T=2.50, V=1.50
0	-1400	CASE	E 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND
) 0	100 19500			OLF: L=1.00, T=1.00, V=1.00
0	-500	CASE	Ξ 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND
)	2600			OLF: L=1.00, T=1.00, V=1.00
0 0	7500 -800	CASE	<u> </u>	NESC MEDIUM DEADEND: 15°, .25" ICE, 4 PSF WIND
)	100			OLF: L=1.65, T=2.50, V=1.50
)	3500	CASE	E 5	NESC HIGH WIND DEADEND: 60°, 0" ICE, 120 MPH WIND
0	-100 100			OLF: L=1.00, T=1.00, V=1.00
0	1500	CASE	E 6	NESC ICE WITH WIND DEADEND: 15°, 1" ICE, 40 MPH WIND
	-100			OLF: L=1.00, T=1.00, V=1.00
)	200 500	CASE	Ξ 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
0	-600		- 40	
)	300	CASE	- 10	STRINGING: -20°, 0" ICE, 2 PSF WIND OLF: L=1.50, T=1.50, V=1.50
, 0	1700 1700	WIRI	e da	
)	300			#9" ALUMOWELD
)	1700 1700			72 KCMIL 61/0 STRAND "NARCISSUS" AAC
)	700			336.4 KCMIL 18/1 STRAND "RAVEN" ACSR
)	300			FION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR
)	600 3			T-XXX27DT-144-CLCB"144 FIBER
[	J	NO	TES	
		1. /	ALL S	TATED LOADS ARE ULTIMATE VALUES AND INCLUDE
RE.				OAD FACTORS AND INSULATOR WEIGHT.
κĖ,				TURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE TANEOUS APPLICATION OF DEAD LOAD, WIND ON THE
			STRUC	TURE, AND WIRE LOADS FOR EACH LOADING CASE.
				TURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE
				REMENTS OF THE STEEL POLE SPECIFICATIONS.
				PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A
				EFACTOR OF 1.0. Cator may propose structural details different
				THOSE SHOWN TO SIMPLIFY FABRICATION.VARIATIONS
				BE SUBMITTED TO ENGINEER IN WRITING.
				UM VANG PLATE THICKNESS = 1/2". Shall be applied in the direction which results in
				OST SEVERE EFFECT.
				EFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5%
				E POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY MBERED TO FALL WITHIN THE DESIGN LIMIT.
		9. N		UM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE
	V1	L1 l		D TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES
	T1"			THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. TRUCTURES SHALL BE GALVANIZED STEEL.
		$V^2_{1/2}$ L2 11. (		T SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
			RAW	
		√2 ∟o (		ACTURER SHALL APPLY POINT LOADS NECESSARY TO E THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING
				POLES, BASE PLATES, ETC.
		~ <b>-</b>  2		
		√2 L2		
		12-T2		
	\	<sup>/3</sup> .L3		
		L-T3		
	\	/3 L3		
		<u>—</u> тз		
	\	/3 L3		
		<b>⊻-</b> T3		
		V7, L7		
		12-T7		
		V4 I A		
		V <sup>8</sup> L8 ↓∕		
		<b>1</b> 8		
		√5 <sub>L5</sub>		
$\lor$	<sup>′9</sup>	V <sub>6 L6</sub>		
,	<sup>9</sup> L9			
		V6 L6		



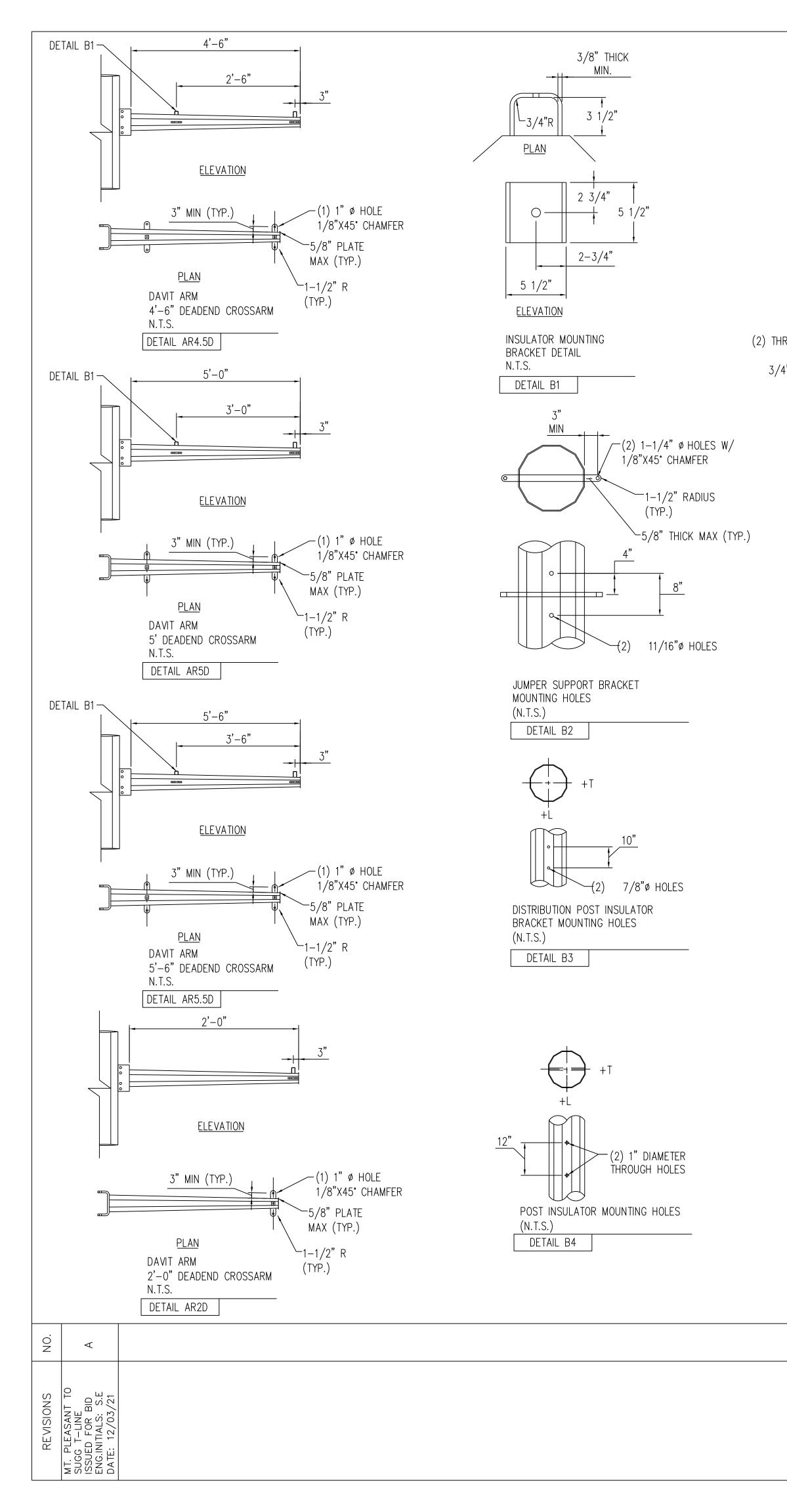
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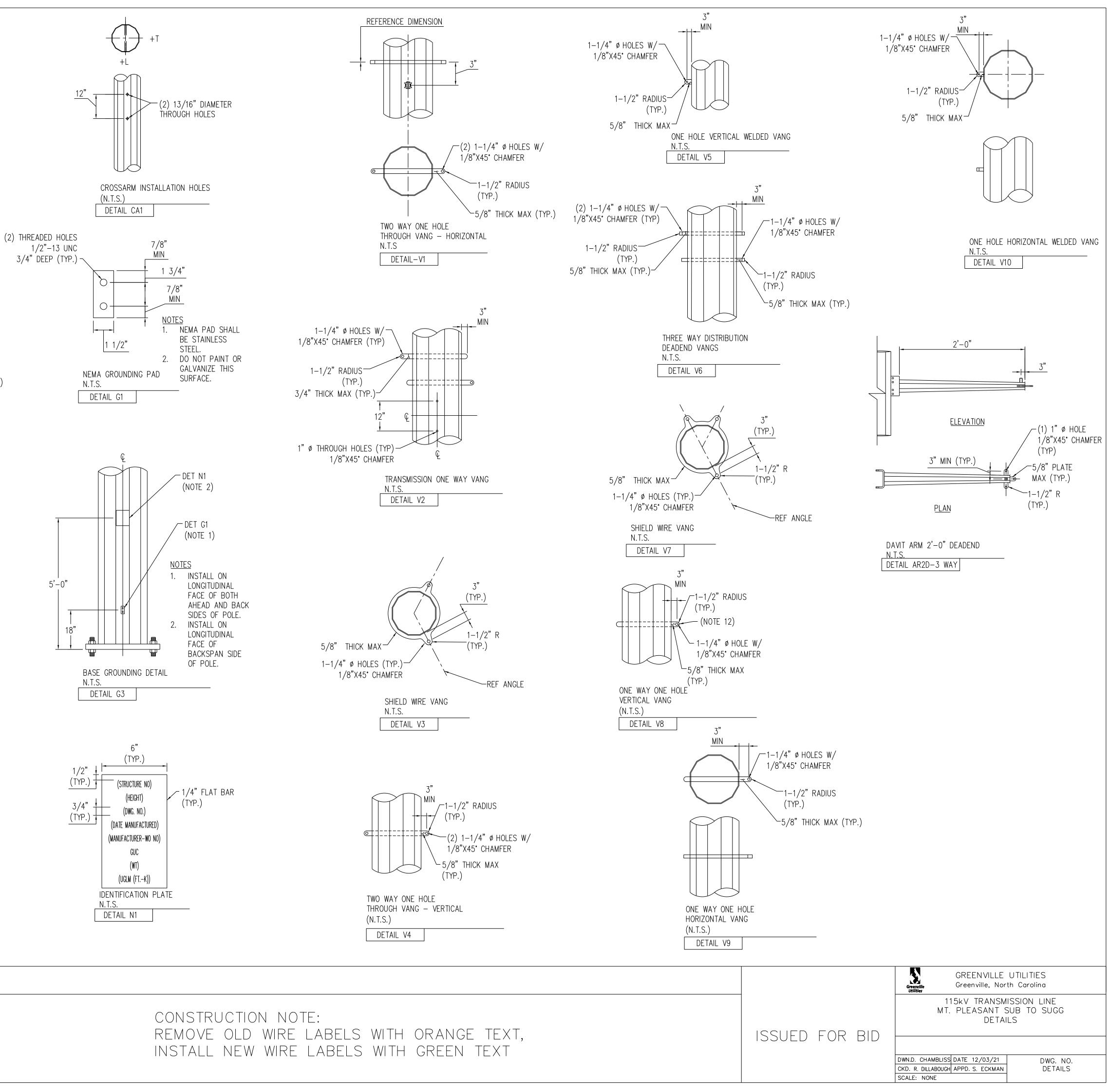
GREENVILLE UTILITIES Greenville, North Carolina

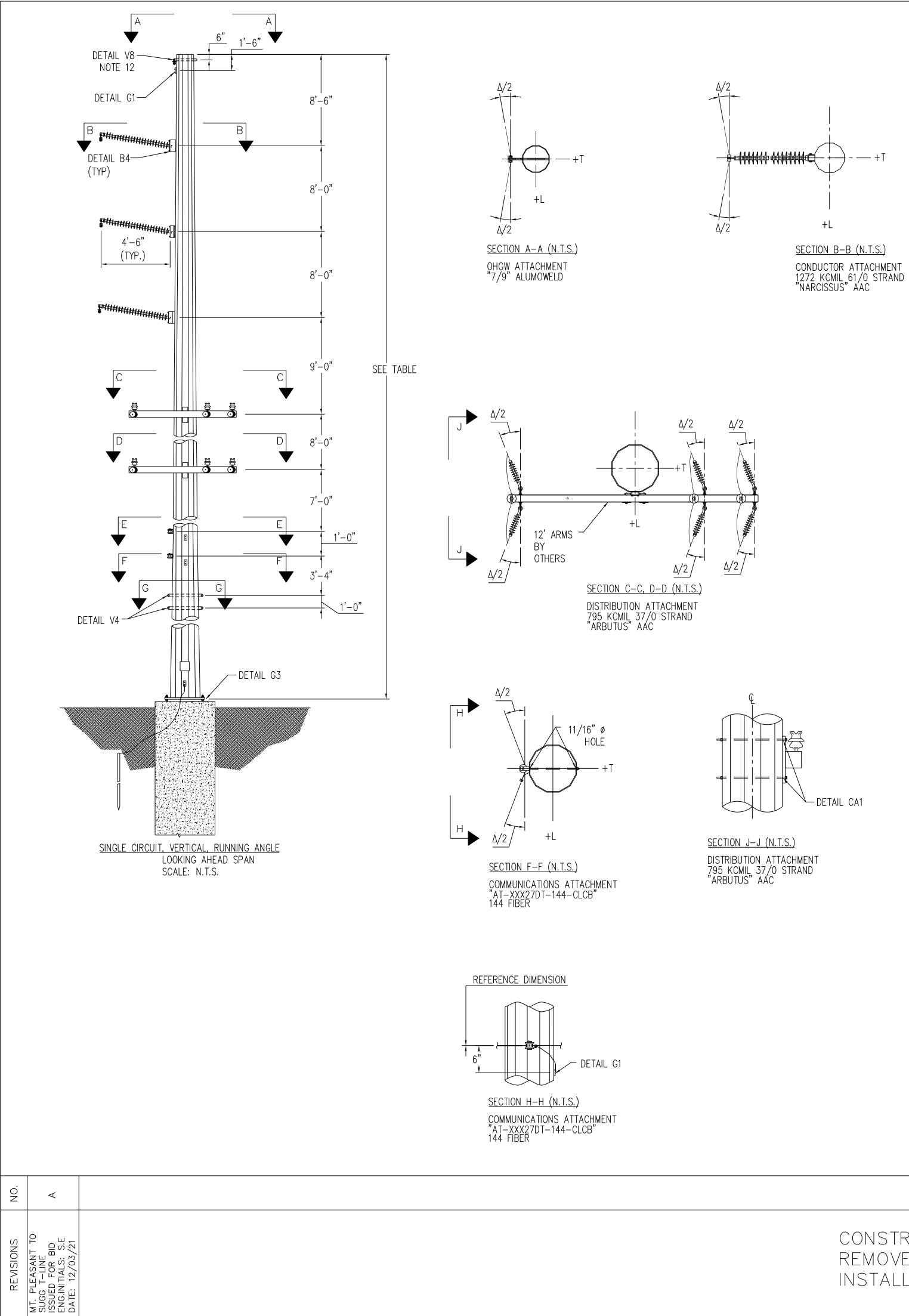
115kv transmission line
MT. PLEASANT SUB TO SUGG
LOAD AND DESIGN
DEADEND 60°-90° WITH UNDERBUILD

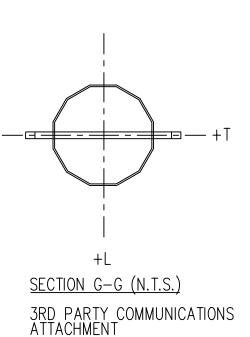
DWN.D. CHAMBLISSDATE 12/03/21CKD. R. DILLABOUGHAPPD. S. ECKMAN SCALE: NONE

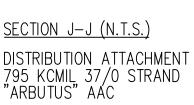
Greenville Utilities

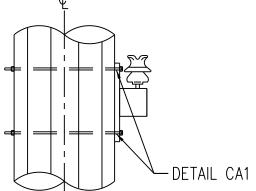










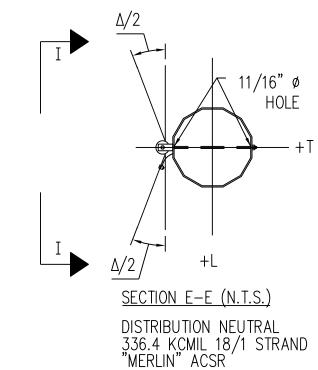


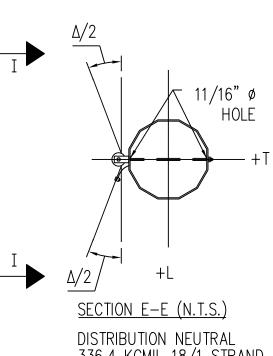


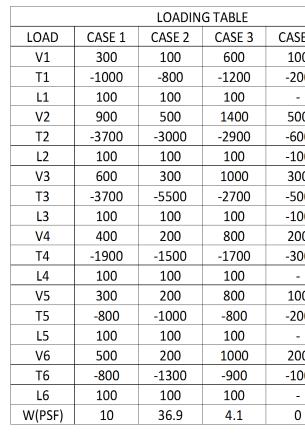
<u>SECTION I-I (N.T.S.)</u>

DISTRIBUTION NEUTRAL 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

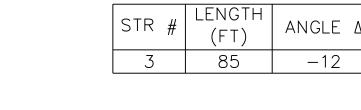
- DETAIL G1







ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.



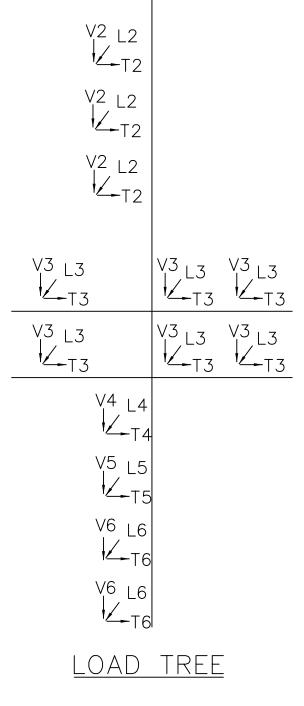
E 7	CASE 9
0	700
00	-1000
	-300
0	1500
00	-2600
00	-200
0	1100
00	-2100
00	-1100
0	900
00	-1400
	-900
0	900
00	-600
	-500
0	1100
00	-700
	-400
	0

LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

<u>WIRE DATA</u> OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 795 KCMIL 37/0 STRAND "ARBUTUS"AAC DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

NOTES:

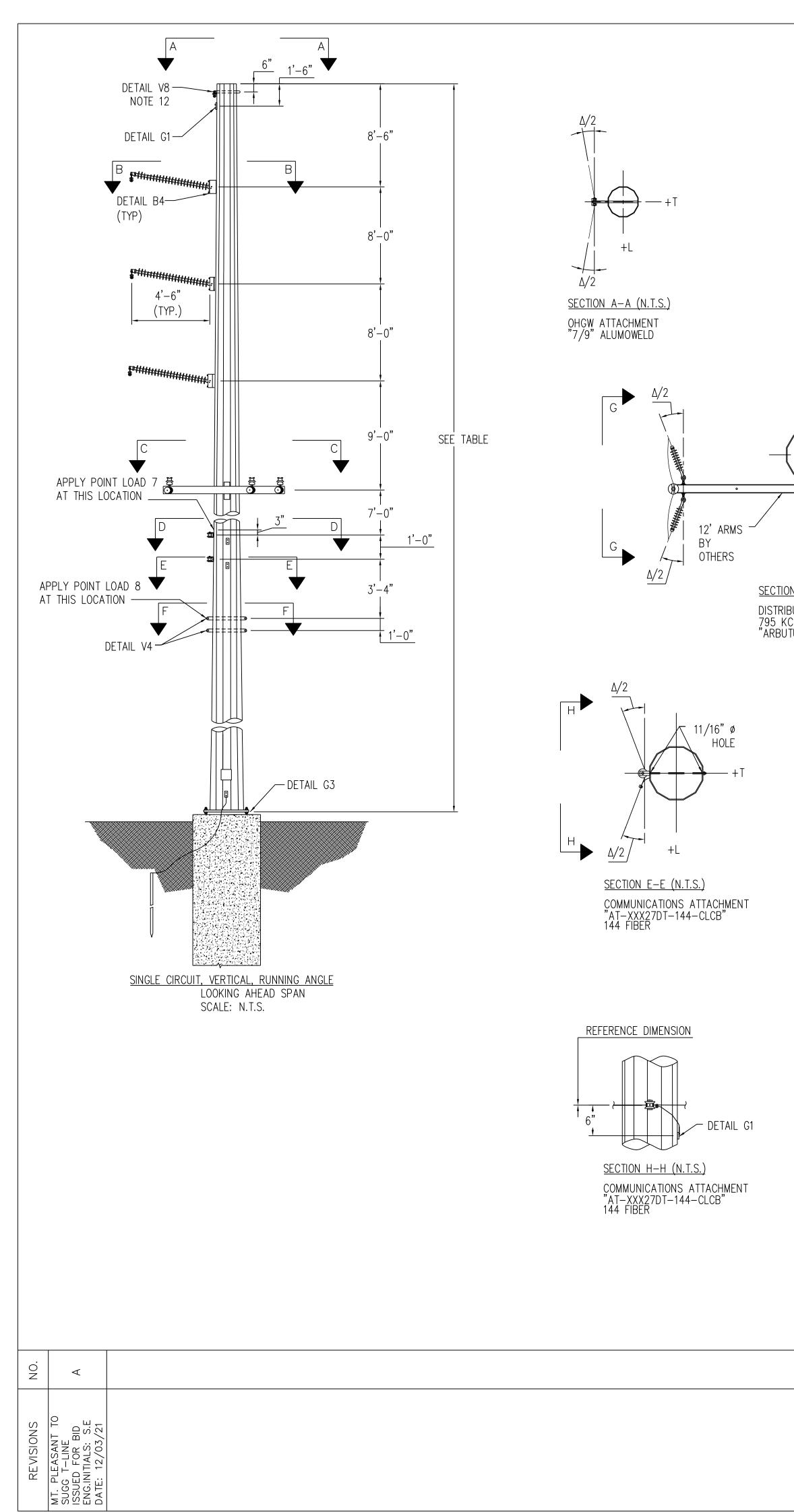
- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
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- 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
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- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

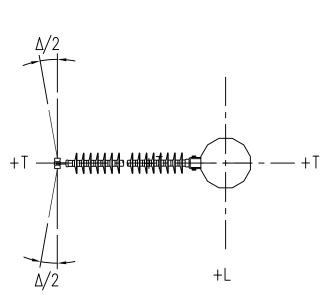


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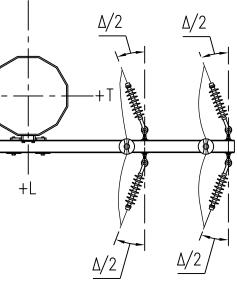
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			Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUED FOR BI	BID		115kV TRANSMI MT. PLEASANT S LOAD AND JNNING ANGLE WI	UB TO SUGG DESIGN	
				BLISS DATE 12/03/21 BOUGH APPD. S. ECKMAN	DWG. NO. RA-15L_2FFD_1-CT

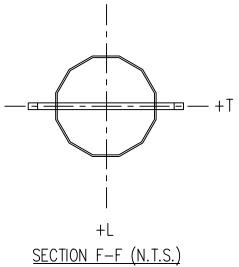




<u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC



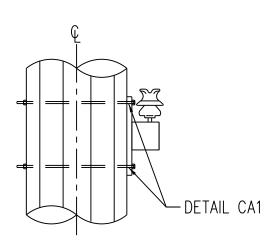
<u>SECTION C-C, (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC



3RD PARTY COMMUNICATIONS ATTACHMENT

	Δ/2
Ι	
	11/16" ø HOLE
	+T
I	$\Delta/2$ +L

<u>SECTION D–D (N.T.S.)</u> DISTRIBUTION NEUTRAL 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR



<u>SECTION G-G (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC

STR #	LENGTH (FT)	ANGLE A
18	80	-10
35	80	-4
40	75	-8
45	75	-15
46	80	-12
65*	75	-6
147*	75	-3

			G TABLE		
LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9
V1	300	100	600	100	700
T1	-1000	-800	-1200	-200	-1000
L1	-100	-100	-100	-	-300
V2	900	500	1400	500	1500
Т2	-3700	-3000	-2900	-600	-2600
L2	-100	-100	-100	-100	-200
V3	600	300	1000	300	1100
Т3	-3700	-5500	-2700	-500	-2100
L3	-100	-100	-100	-100	-1100
V4	400	200	800	200	900
T4	-1900	-1500	-1700	-300	-1400
L4	-100	-100	-100	-	-900
V5	300	200	800	100	900
T5	-800	-1000	-800	-200	-600
L5	-100	-100	-100	-	-500
V6	500	200	1000	200	1100
Т6	-800	-1300	-900	-100	-700
L6	-100	-100	-100	-	-400
V7	800	300	1800	300	300
T7	-800	-1400	-1100	-100	-800
L7	-100	-200	-200	-100	-200
V8	900	300	2000	300	300
Т8	-1900	-2500	-2100	-400	-1700
L8	-300	-300	-400	-100	-300
W(PSF)	10	36.9	4.1	0	0

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

\* APPLY POINT LOAD 7 AND 8 TO STRUCTURES 65 AND 147 ONLY.

REFERENCE DIMENSION

∕− DETAIL G1

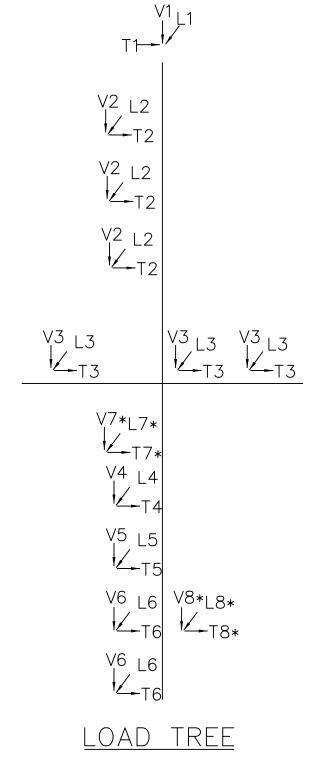
<u>SECTION I–I (N.T.S.)</u> DISTRIBUTION NEUTRAL 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

CONSTRUCTION NOTE: REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT

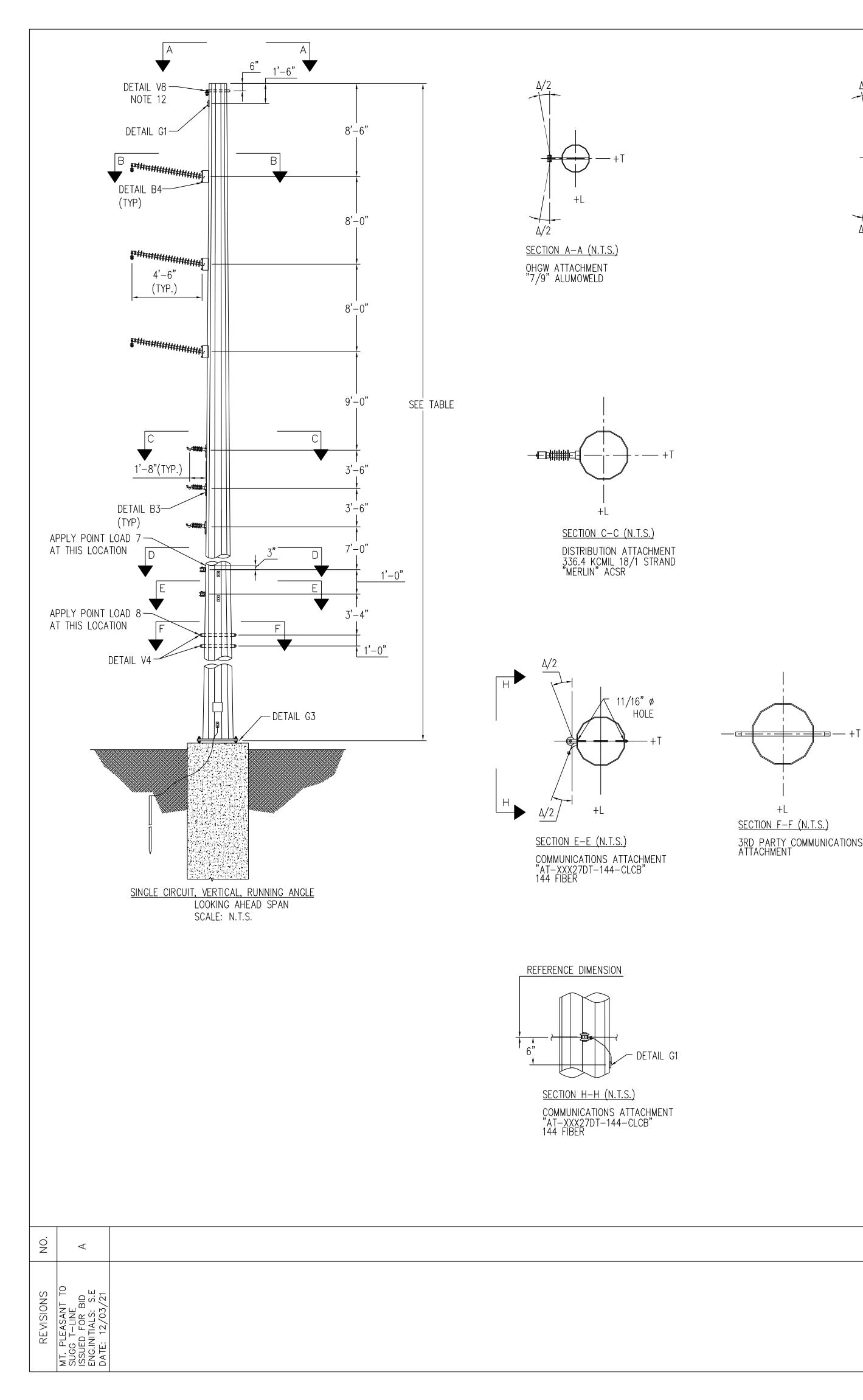
LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1"ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

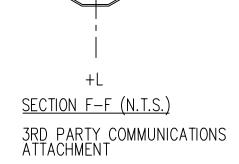
WIRE DATA OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 795 KCMIL 37/0 STRAND "ARBUTUS"AAC DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT.
  2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2".
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
   ALL STRUCTURES SHALL BE CALVANIZED STEEL
- ALL STRUCTURES SHALL BE GALVANIZED STEEL.
   ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

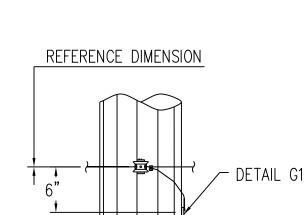


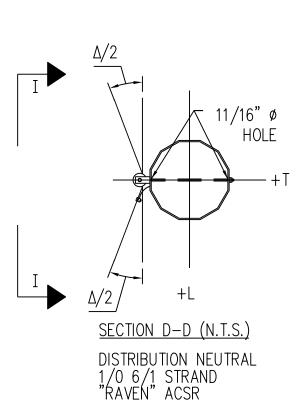
		Greenville Utilities	GREENVILLE Greenville, Nort	
	115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN RUNNING ANGLE WITH UNDERBUILD			
ISSUED FOF	K BID	DWN.D. CHAMBLISS CKD. R. DILLABOUGH SCALE: NONE		DWG. NO. RA-15L_FFD_1-CT

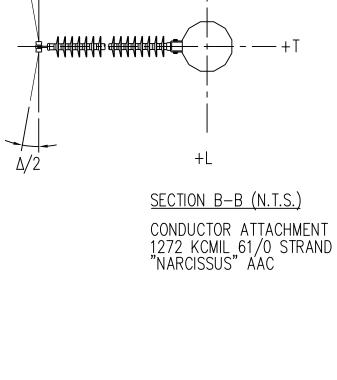




<u>SECTION I-I (N.T.S.)</u> DISTRIBUTION NEUTRAL 1/0 6/1 STRAND "RAVEN" ACSR







 $\Delta/2$ 

STR #	LENGTH (FT)	ANGLE A
114	90	-5
115	90	-5
140	80	-14
141*	80	-9

		LOADIN	G TABLE	
LOAD	CASE 1	CASE 2	CASE 3	
V1	300	100	600	
T1	-1000	-800	-1200	
L1	-100	-100	-100	
V2	900	500	1400	
Т2	-3700	-3000	-2900	
L2	-100	-100	-100	
V3	600	300	1000	
Т3	-3700	-5500	-2700	
L3	-100	-100	-100	
V4	400	200	800	
T4	-1900	-1500	-1700	
L4	-100	-100	-100	
V5	300	200	800	
T5	-800	-1000	-800	
L5	-100	-100	-100	
V6	500	200	1000	
Т6	-800	-1300	-900	
L6	-100	-100	-100	
V7	600	200	1400	
T7	-400	-800	-400	
L7	-500	-700	-800	
V8	700	300	1500	
Т8	-700	-1100	-600	
L8	-1500	-1500	-1600	
W(PSF)	10	36.9	4.1	

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

\* APPLY POINT LOAD 7 AND 8 TO STRUCTURE 141 ONLY.

CASE 7	CASE 9
100	700
-200	-1000
-	-300
500	1500
-600	-2600
-100	-200
300	1100
-500	-2100
-100	-1100
200	900
-300	-1400
-	-900
100	900
-200	-600
-	-500
200	1100
-100	-700
-	-400
200	200
-100	-200
-100	-700
300	300
-100	-300
-500	-1500
0	0

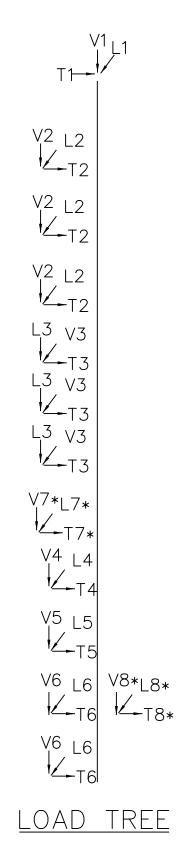
# LOAD CASES CASE 1 NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50 CASE 2 NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00 CASE 3 NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00 CASE 7 DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00

CASE 9 UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

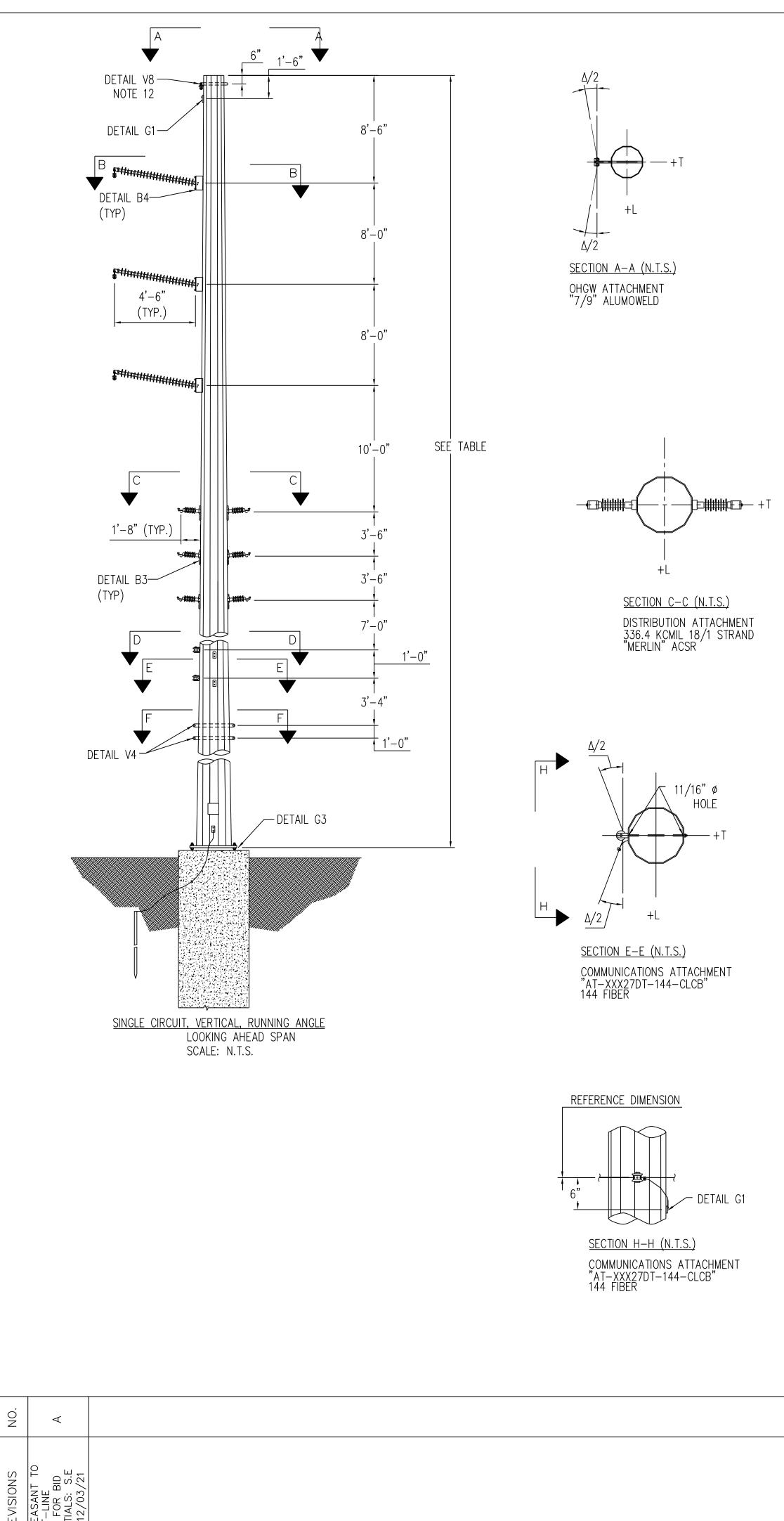
# WIRE DATA

OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL.
- 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.



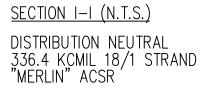
			E UTILITIES orth Carolina
ISSUFD FOR		MT. PLEASANT LOAD AN	SMISSION LINE SUB TO SUGG D DESIGN WITH UNDERBUILD
ISSUED FUR	DID	DWN.D. CHAMBLISS DATE 12/03/21 CKD. R. DILLABOUGH APPD. S. ECKMA SCALE: NONE	

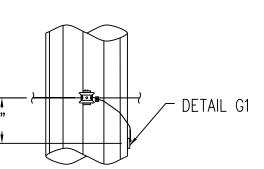


MT. PLE SUGG T ISSUED ENG.INI DATE:

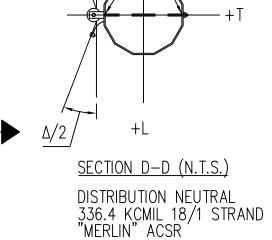


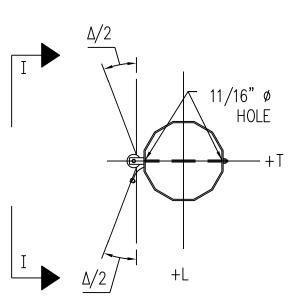
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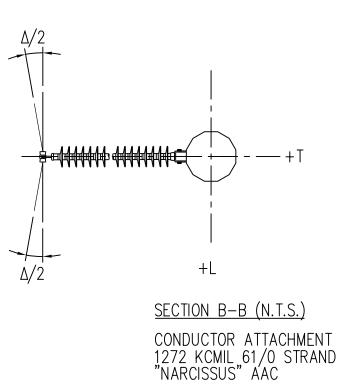




REFERENCE DIMENSION







STR #	LENGTH (FT)	ANGLE A
61	90	-6

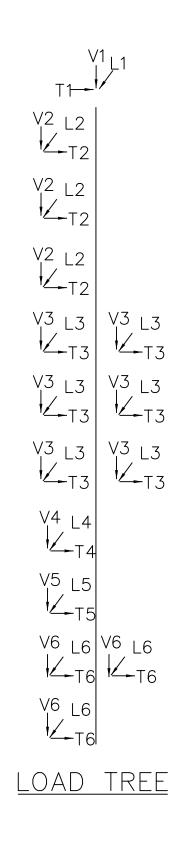
LOADING TABLE							
LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9		
V1	300	100	600	100	700		
T1	-1000	-800	-1200	-200	-1000		
L1	100	100	100	-	-300		
V2	900	500	1400	500	1500		
Т2	-3700	-3000	-2900	-600	-2600		
L2	100	100	100	-100	-200		
V3	600	300	1000	300	1100		
Т3	-3700	-5500	-2700	-500	-2100		
L3	100	100	100	-100	-1100		
V4	400	200	800	200	900		
T4	-1900	-1500	-1700	-300	-1400		
L4	100	100	100	-	-900		
V5	300	200	800	100	900		
T5	-800	-1000	-800	-200	-600		
L5	100	100	100	-	-500		
V6	500	200	1000	200	1100		
Т6	-800	-1300	-900	-100	-700		
L6	100	100	100	-	-400		
W(PSF)	10	36.9	4.1	0	0		

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

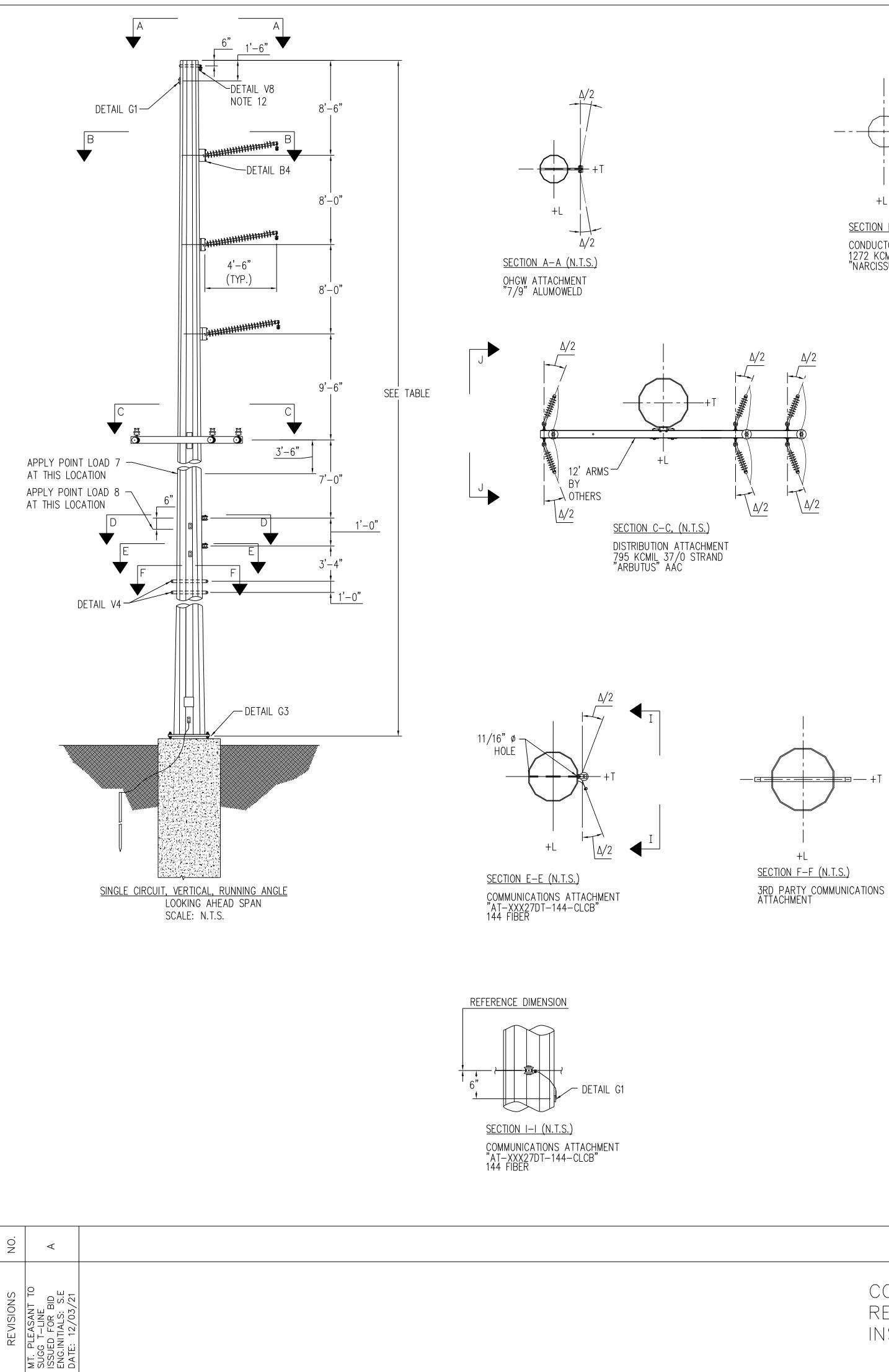
LOAD	<u>CASES</u>
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

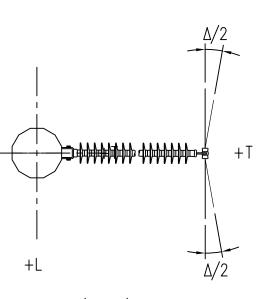
<u>WIRE DATA</u> OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE
- OVERLOAD FACTORS AND INSULATOR WEIGHT. 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
- DRAWINGS.
- 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

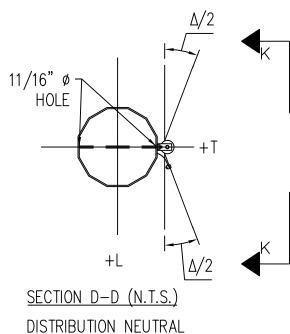


		Greenville Utilities	GREENVILLE Greenville, Nor	
ISSUED FOR BID	RI	115kV TRANSM MT. PLEASANT S LOAD AND UNNING ANGLE W	SUB TO SUGG DESIGN	
			MBLISS DATE 12/03/21 ABOUGH APPD. S. ECKMAN	DWG. NO. RA-15L_Vert_1-CT_STR-61





<u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC

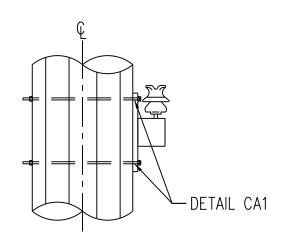


336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

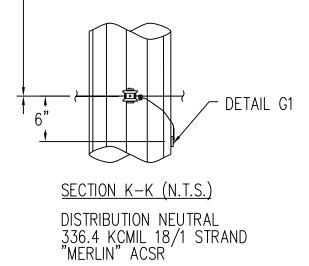
STR #	HEIGHT (FT)	ANGLE A
11	75	7

LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9
V1	200	200	600	100	600
T1	1100	900	1200	200	1100
L1	-100	-100	-100	-100	-300
V2	900	500	1300	400	1400
T2	4300	3600	3400	1000	3100
L2	-100	-100	-100	-100	-200
V3	500	300	1000	300	1000
Т3	3900	5800	2800	600	2300
L3	-100	-100	-100	-100	-1300
V4	400	200	900	200	900
T4	2000	1700	1800	400	1600
L4	-100	-100	-100	-100	-1000
V5	300	100	800	100	900
T5	900	1200	900	200	700
L5	-100	-100	-100	-100	-700
V6	500	200	1000	200	1100
T6	900	1600	1000	100	700
L6	-100	-100	-100	-	-600
V7	500	200	1600	200	200
T7	-1900	-900	-1500	-600	-1600
L7	-400	-200	-300	-100	-300
V8	500	200	1600	200	200
Т8	-1900	-900	-1500	-600	-1600
L8	-400	-200	-300	-100	-300
W(PSF)	10	36.9	4.1	0	0

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.



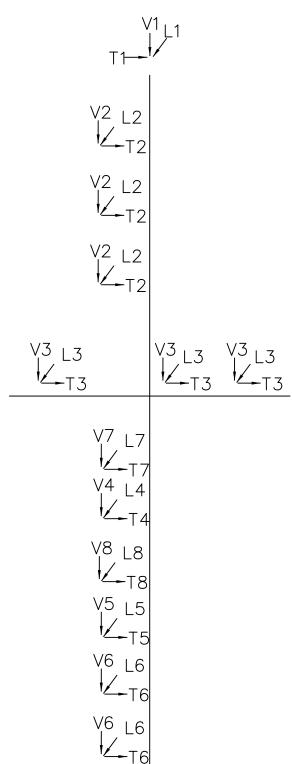
<u>SECTION J-J (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC REFERENCE DIMENSION



CONSTRUCTION NOTE: REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT

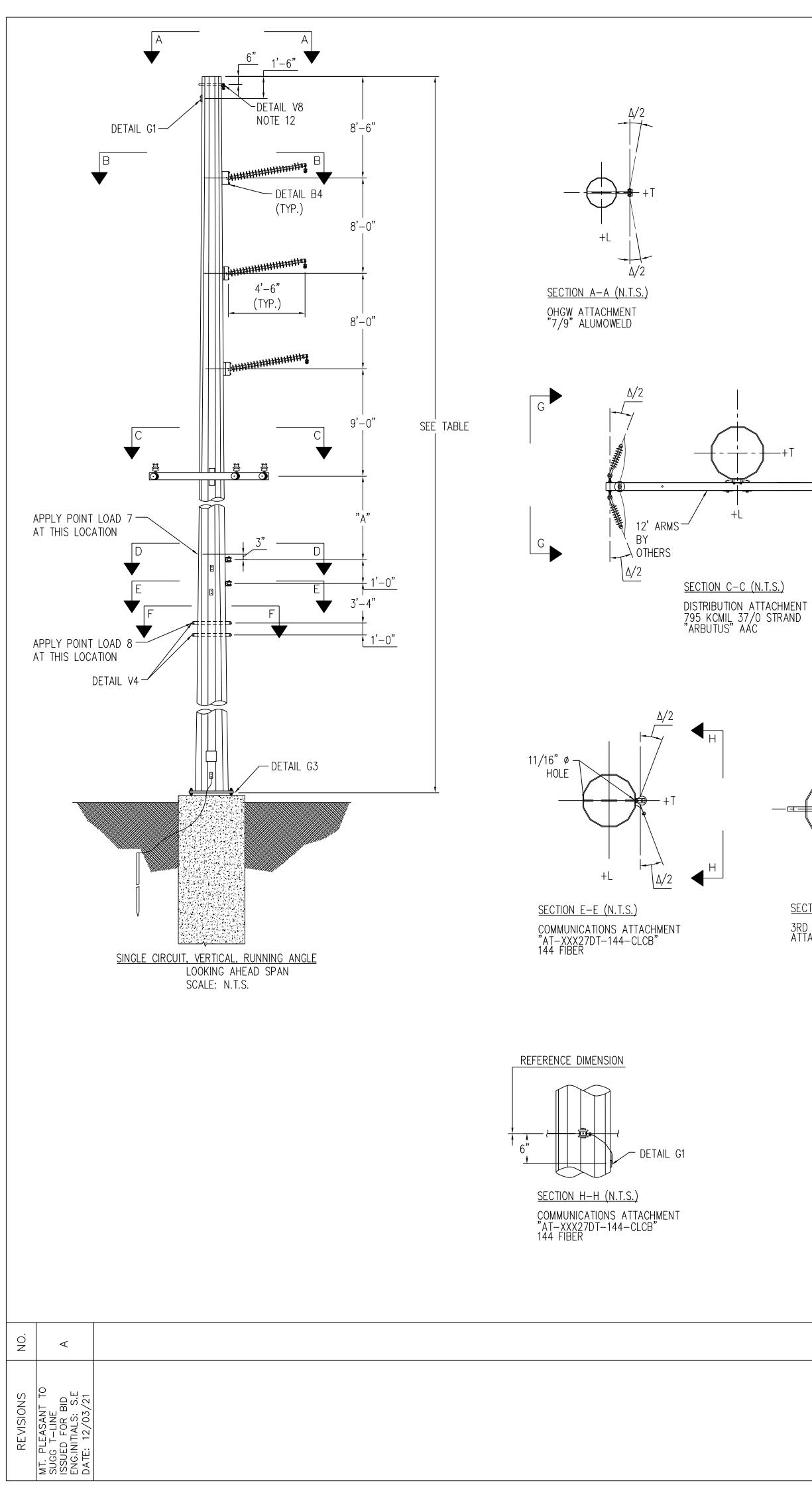
LOAD (	<u>CASES</u>
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10
115KV: 12 12.47kV: 7 DISTRIBUT	<u>TA</u> #9" ALUMOWELD 72 KCMIL 61/0 STRAND "NARCISSUS" AAC 795 KCMIL 37/0 STRAND "ARBUTUS"AAC FION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR F—XXX27DT—144—CLCB" 144 FIBER
NOTES	

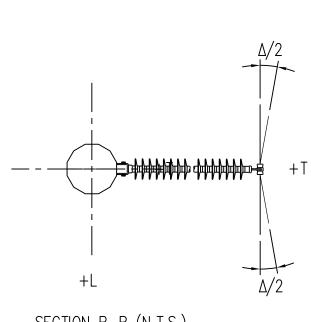
- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE OVERLOAD FACTORS AND INSULATOR WEIGHT.
- 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0. 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT
- THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING. 6. MINIMUM VANG PLATE THICKNESS = 1/2".
- 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE.
- 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL. 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
- DRAWINGS. 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.



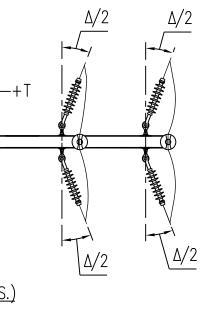
# LOAD TREE

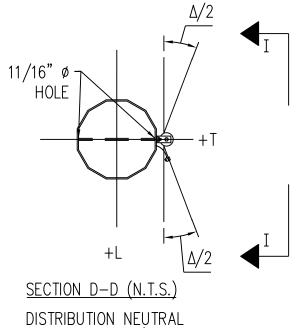
		Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUED FC	)r bid		115kV TRANSMI IT. PLEASANT S LOAD AND ING ANGLE 10°	SUB TO SUGG
			LISS DATE 12/03/21 UGH APPD. S. ECKMAN	DWG. NO. RA-15R_FFD_1.5-CT





# <u>SECTION B-B (N.T.S.)</u> CONDUCTOR ATTACHMENT 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC





HEIGHT DIMENSION

(FT)

75

90

75

85

85

80

80

80

80

80

"A" (FT)

7

7

7

7

7

7

10

7

7

7

ANGLE

8

10

4

4

10

15

7

10

7

6

STR #

13

21

38

42

49

50

100

101\*

102

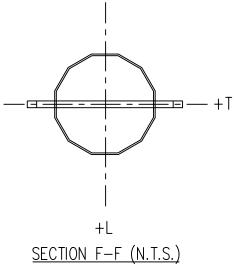
145\*

DISTRIBUTION NEUTRAL 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

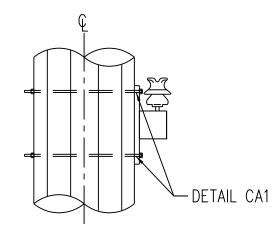
	1	LOADIN			
LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9
V1	300	100	600	100	700
T1	-1000	-800	-1200	-200	-1000
L1	-100	-100	-100	-	-300
V2	900	500	1400	500	1500
T2	-3700	-3000	-2900	-600	-2600
L2	-100	-100	-100	-100	-200
V3	600	300	1000	300	1100
Т3	-3700	-5500	-2700	-500	-2100
L3	-100	-100	-100	-100	-1100
V4	400	200	800	200	900
T4	-1900	-1500	-1700	-300	-1400
L4	-100	-100	-100	-	-900
V5	300	200	800	100	900
T5	-800	-1000	-800	-200	-600
L5	-100	-100	-100	-	-500
V6	500	200	1000	200	1100
Т6	-800	-1300	-900	-100	-700
L6	-100	-100	-100	-	-400
V7	500	100	1600	100	100
T7	600	700	700	100	500
L7	-100	-100	-100	-100	-100
V8	1100	400	2500	400	400
Т8	1900	2100	1700	600	1400
L8	-300	-300	-300	-100	-300
W(PSF)	10	36.9	4.1	0	0

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

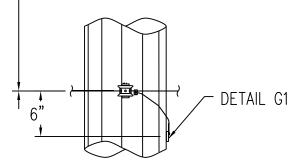
\* APPLY POINT LOAD 7 AND 8 TO STRUCTURE 101 AND 145 ONLY.



3RD PARTY COMMUNICATIONS ATTACHMENT



<u>SECTION G-G (N.T.S.)</u> DISTRIBUTION ATTACHMENT 795 KCMIL 37/0 STRAND "ARBUTUS" AAC REFERENCE DIMENSION



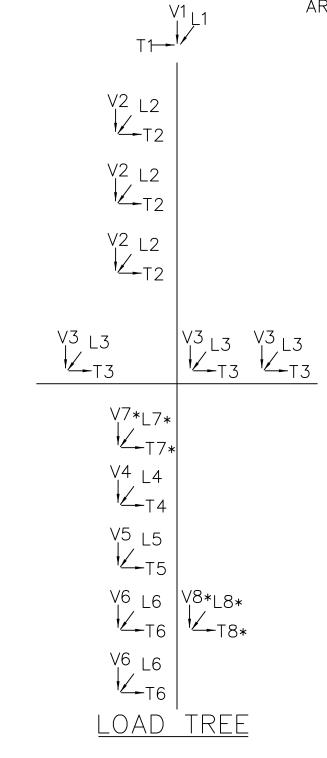
<u>SECTION I-I (N.T.S.)</u> DISTRIBUTION NEUTRAL 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR

CONSTRUCTION NOTE: REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT

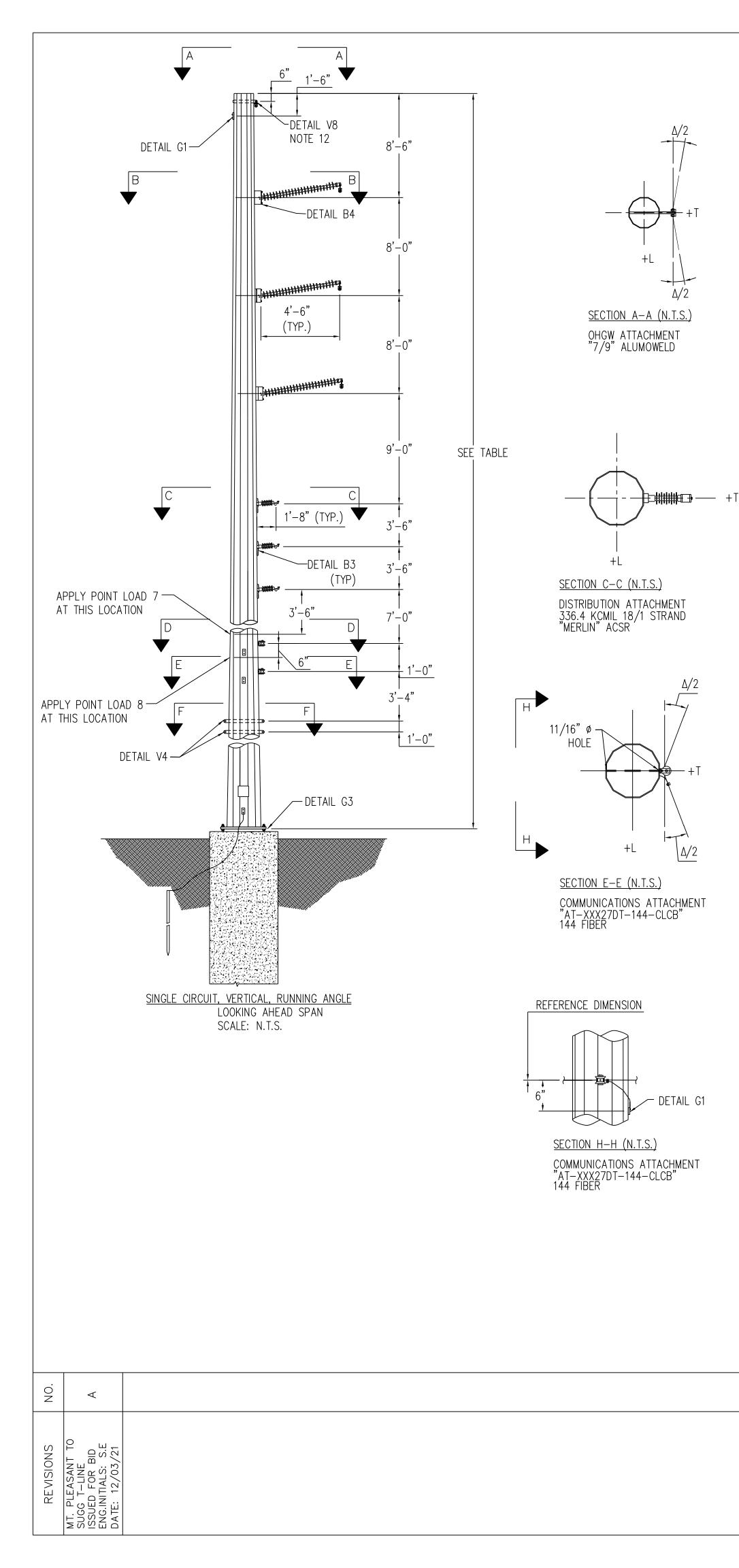
LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 3	NESC ICE WITH WIND: 15°, 1" ICE, 40 MPH WIND OLF: L=1.00, T=1.00, V=1.00
CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

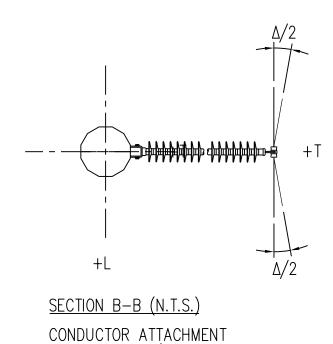
WIRE DATA OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 795 KCMIL 37/0 STRAND "ARBUTUS"AAC DISTRIBUTION NEUTRAL: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

- 1. ALL STATED LOADS ARE ULTIMATE VALUES AND INCLUDE OVERLOAD FACTORS AND INSULATOR WEIGHT.
- 2. STRUCTURE AND ATTACHMENTS SHALL BE DESIGNED FOR THE SIMULTANEOUS APPLICATION OF DEAD LOAD, WIND ON THE STRUCTURE, AND WIRE LOADS FOR EACH LOADING CASE.
- 3. STRUCTURE SHALL BE DESIGNED SELF SUPPORTING, GUYS ARE NOT PERMITTED. STRUCTURE SHALL MEET ALL TECHNICAL REQUIREMENTS OF THE STEEL POLE SPECIFICATIONS.
- 4. WIND PRESSURES SHOWN ON LOAD WORKSHEET ARE BASED ON A SHAPE FACTOR OF 1.0.
- 5. FABRICATOR MAY PROPOSE STRUCTURAL DETAILS DIFFERENT THAN THOSE SHOWN TO SIMPLIFY FABRICATION. VARIATIONS SHALL BE SUBMITTED TO ENGINEER IN WRITING.
- 6. MINIMUM VANG PLATE THICKNESS = 1/2". 7. WIND SHALL BE APPLIED IN THE DIRECTION WHICH RESULTS IN THE MOST SEVERE EFFECT.
- 8. THE DEFLECTION AT THE POLE TOP SHALL BE LIMITED TO 1.5% OF THE POLE HEIGHT UNDER THE DEFLECTION CASE. POLES MAY BE CAMBERED TO FALL WITHIN THE DESIGN LIMIT.
- 9. MAXIMUM DEFLECTION AT TOP OF THE STRUCTURE SHALL BE LIMITED TO 10% OF STRUCTURE HEIGHT UNDER ALL LOAD CASES WITH THE EXCEPTION TO THE 60°F NO WIND LOAD CASE. 10. ALL STRUCTURES SHALL BE GALVANIZED STEEL.
- 11. ORIENT SINGLE SIDED VANGS FOR HARDWARE SHOWN ON
- DRAWINGS. 12. MANUFACTURER SHALL APPLY POINT LOADS NECESSARY TO CREATE THE MOST SEVERE EFFECTS ON ALL MEMBERS INCLUDING ARMS, POLES, BASE PLATES, ETC.

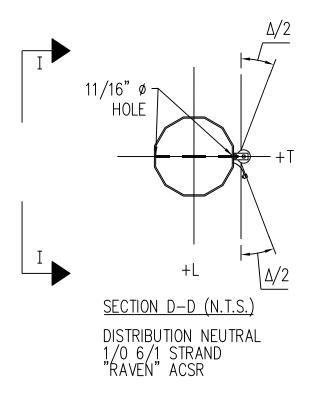


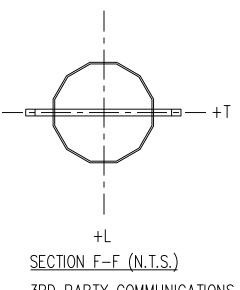
			Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUED	FOR	BID		115kV TRANSM MT. PLEASANT S LOAD AND NNING ANGLE WI	SUB TO SUGG DESIGN
				BLISS DATE 12/03/21 OUGH APPD. S. ECKMAN	DWG. NO. RA-15R_FFD_1-CT





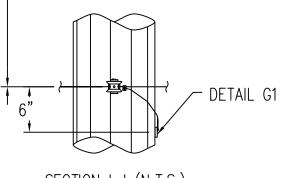
1272 KCMIL 61/0 STRAND "NARCISSUS" AAC





3RD PARTY COMMUNICATIONS ATTACHMENT

REFERENCE DIMENSION



<u>SECTION I-I (N.T.S.)</u> DISTRIBUTION NEUTRAL 1/0 6/1 STRAND "RAVEN" ACSR

STR #	HEIGHT (FT)	ANGLE A
142	80	4

		LOADIN	G TABLE	-	
LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9
V1	200	200	600	100	600
T1	1100	900	1200	200	1100
L1	-100	-100	-100	-100	-300
V2	900	500	1300	400	1400
Т2	4300	3600	3400	1000	3100
L2	-100	-100	-100	-100	-200
V3	500	300	1000	300	1000
Т3	3900	5800	2800	600	2300
L3	-100	-100	-100	-100	-1300
V4	400	200	900	200	900
T4	2000	1700	1800	400	1600
L4	-100	-100	-100	-100	-1000
V5	300	100	800	100	900
T5	900	1200	900	200	700
L5	-100	-100	-100	-100	-700
V6	500	200	1000	200	1100
Т6	900	1600	1000	100	700
L6	-100	-100	-100	-	-600
V7	600	200	2100	200	200
T7	1800	1000	1300	500	1000
L7	-1000	-500	-700	-300	-600
V8	600	200	2100	200	200
Т8	1100	700	900	100	700
L8	-600	-300	-400	-100	-400
W(PSF)	10	36.9	4.1	0	0

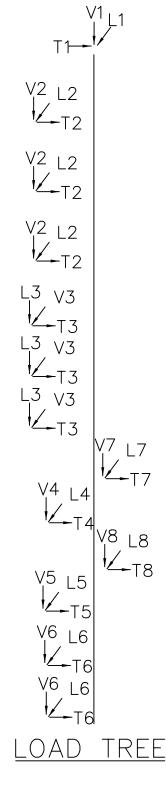
ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

CONSTRUCTION NOTE: REMOVE OLD WIRE LABELS WITH ORANGE TEXT, INSTALL NEW WIRE LABELS WITH GREEN TEXT

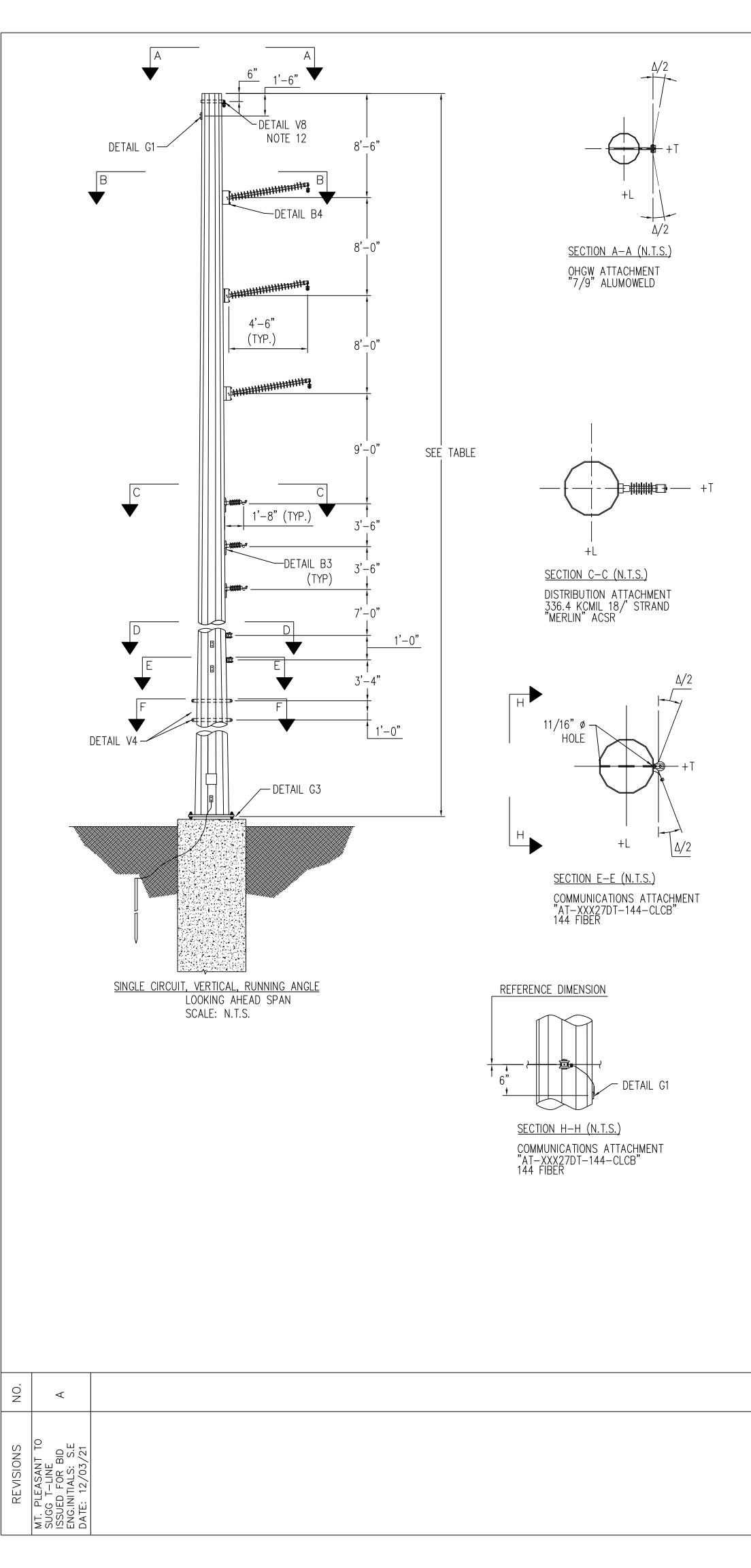
LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
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CASE 9	UNBALANCED ICE: 32 DEGREES, 1" ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

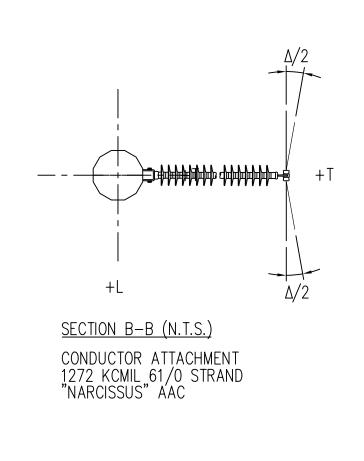
<u>WIRE DATA</u> OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

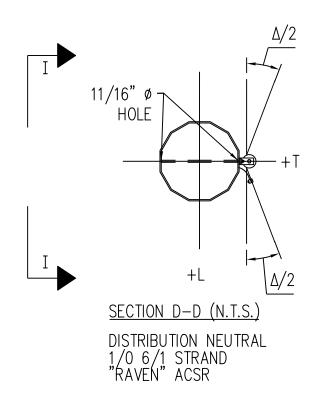
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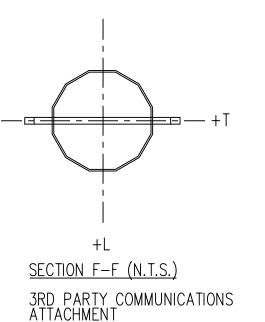


			Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUED	FOR	BID		115kV TRANSMI 1T. PLEASANT S LOAD AND ING ANGLE 10°	SUB TO SUGG
				ISS DATE 12/03/21 UGH APPD. S. ECKMAN	DWG. NO. RA-15R_Vert_1.5-CT

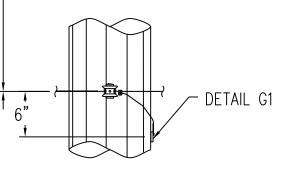








REFERENCE DIMENSION



<u>SECTION I–I (N.T.S.)</u> DISTRIBUTION NEUTRAL 1/0 6/1 STRAND "RAVEN" ACSR

		LOADIN	G TABLE	
LOAD	CASE 1	CASE 2	CASE 3	CA
V1	200	200	600	-
T1	1100	900	1200	
L1	-100	-100	-100	-
V2	900	500	1300	2
T2	4300	3600	3400	1
L2	-100	-100	-100	-
V3	500	300	1000	3
Т3	3900	5800	2800	(
L3	-100	-100	-100	-
V4	400	200	900	-
T4	2000	1700	1800	4
L4	-100	-100	-100	-
V5	300	100	800	-
T5	900	1200	900	-
L5	-100	-100	-100	-
V6	500	200	1000	
T6	900	1600	1000	-
L6	-100	-100	-100	
W(PSF)	10	36.9	4.1	

HEIGHT

(FT)

85

ANGLE **A** 

7

STR #

143

ALL LOADS ARE IN LBS, ARE ULTIMATE, AND INCLUDE ALL OVERLOAD FACTORS. "W" REPRESENTS WIND PRESSURE (psf) TO BE APPLIED TO STRUCTURE.

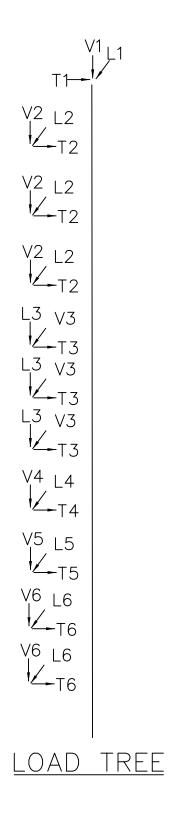
SE 7	CASE 9
.00	600
200	1100
.00	300
00	1400
000	3100
.00	200
800	1000
500	2300
.00	1300
200	900
00	1600
.00	1000
.00	900
200	700
.00	700
200	1100
.00	700
-	600
0	0

LOAD	CASES
CASE 1	NESC MEDIUM: 15°, .25" ICE, 4 PSF WIND OLF: L=1.65, T=2.50, V=1.50
CASE 2	NESC HIGH WIND: 60°, 0" ICE, 120 MPH WIND OLF: L=1.00, T=1.00, V=1.00
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CASE 7	DEFLECTION: 60 DEGREES, NO ICE, NO WIND, FINAL OLF: L=1.00, T=1.00, V=1.00
CASE 9	UNBALANCED ICE: 32 DEGREES, 1"ICE, NO WIND OLF: L=1.10, T=1.10, V=1.10

WIRE DATA OHGW: "7#9" ALUMOWELD 115KV: 1272 KCMIL 61/0 STRAND "NARCISSUS" AAC 12.47kV: 336.4 KCMIL 18/1 STRAND "MERLIN" ACSR DISTRIBUTION NEUTRAL: 1/0 6/1 STRAND "RAVEN" ACSR ADSS: "AT-XXX27DT-144-CLCB" 144 FIBER

<u>NOTES:</u>

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		Greenville Utilities	GREENVILLE Greenville, Nort	
ISSUED FOR BID	RID	115kV TRANSMISSION LINE MT. PLEASANT SUB TO SUGG LOAD AND DESIGN RUNNING ANGLE 10° WITH UNDERBUILD		
ISSULD FOR			S DATE 12/03/21 H APPD. S. ECKMAN	DWG. NO. RA-15R_Vert_1-CT

# Attachment C, Structure Weights and Costs

[Balance of page left blank intentionally]

				Equivalent Structu		
STR Class	STR Height	Total	Structure Weight (lb)	Structure Cost	Total Structure Weight (lb)	Total Structure Cost
H4	70	4	-	\$		\$
	75	-	-	\$		\$
	80	-	-	\$		\$
	85	-	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
H5	70	2	-	\$		\$
	75	-	-	\$		\$
	80	-	-	\$		\$
пэ	85	-	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
	70	13	-	\$		\$
	75	4	-	\$		\$
H6	80	11	-	\$		\$
Hb	85	2	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
	70	3	-	\$		\$
	75	5	-	\$		\$
H7	80	3	-	\$		\$
117	85	7	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
	70	2	-	\$		\$
	75	9	-	\$		\$
H8	80	6	-	\$		\$
110	85	7	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
	70	1	-	\$		\$
	75	1	-	\$		\$
H9	80	5	-	\$		\$
113	85	1	-	\$		\$
	90	-	-	\$		\$
	95	-	-	\$		\$
	70	1	-	\$		\$
	75	4	-	\$		\$
H10±	80	13	-	\$		\$
H10+	85	6	-	\$		\$
	90	5	-	\$		\$
	95	1	-	\$		\$
				Totals		\$-

Engineered Structures					
STR #	STR Height	Structure Weight (Ib)	Total Structure Cost		
1	80		\$		
3	85		\$		
4	90		\$		
5	95		\$		
6	80		\$		
8	80		\$		
11	75		\$		
13	75		\$		
18	80		\$		
21	90		\$		
22	100		\$		
23	95		\$		
25	70		\$		
35	80		\$		
38	75		\$		
40	75		\$		
42	85		\$		
45	75		\$		
46	80		\$		
49	85		\$		
50	80		\$		
52	95		\$		
55	95		\$		
56	95		\$		
61	90		\$		
62	105		\$		
63	105		\$		
64	75		\$		
65	75		\$		
75	80		\$		
85	80		\$		
99	85		\$		
100	80		\$		
101	80		\$		
102	80		\$		
114	90		\$		
115	90		\$		
132	75		\$		
140	80		\$		
141	80		\$		
142	80		\$		
143	85		\$		
145	80		\$		
147	75		\$		
151	80		\$		
153	85		\$		
154	95		\$		
	Totals		\$-		

# Attachment D, Bid Summary-Design Information, and Weights

			POLE D	ESIGN							
DESC	RIPTION				STRUC	<b>FURE HE</b>	IGHT				
DIAMETER TAPE	ER (IN/FT), 1ST SECT.										
	ER (IN/FT), 2ND SECT.										
BOTTOM DIAME	TER										
TOP DIAMETER											
CROSS SECTIO	N TYPE										
MATERIAL THIC	KNESS (IN),1ST SEC										
MATERIAL THIC	KNESS (IN),2ND SEC										
ASTM	MATERIAL										
	GRADE										
GOVERNING LO											
MAX. MOMENT A	AT GROUNDLINE										
MAX. SHEAR AT		<b>_</b>				ļ					
	D AT GROUNDLINE		ļ	ļ	ļ				ļ		
MAX. LOAD IN G			ļ	ļ	ļ	ļ			ļ		
ANCHOR BOLTS	SIZE/SPACING										
	LENGTH										
	CAGE DIAMETER										
			ARM DE	ESIGN							
DESC	RIPTION				ARM TY	PE AND	DATA				
		A	В	С	D	E	F			OHGW	
TAPER (IN/FT)			_								
END DIAMETER	(IN)										
DIAMETER AT P											
CROSS SECTIO											
ASTM	MATERIAL										
	GRADE										
GOVERNING LO	AD CASE(S)										
MOMENT AT THI	E POLE (KIP-FT)										
GOVERNING LO	AD CASE(S)										
MOMENT AT THI	E POLE (KIP-FT)										
			SUMMA	RY							
ITEMS	<u>,</u>	1			STRUC	<b>FURE HE</b>					
			1	1	T	I UKE HE I			I	<b>I</b>	
WEIGHT OF ARM		+									
WEIGHT OF ARK		+	1		1	<u> </u>			1		
WEIGHT OF FOL		+									
	PER STRUCTURE	1									
TOTAL COST PE											
NUMBER OF STRUCTURES											
TOTAL WEIGHTS											
TOTAL COSTS		1	1						1		
COMMENTS			1		t i		· ·		1		
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# Attachment E, Previous Questions and Answers

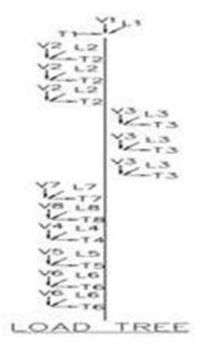
[Balance of page left blank intentionally]

- 1. Section length- On the drawings, below note is mentioned-
  - 10. POLE DESIGN AND FABRICATION SHALL INCLUDE PROVISIONS FOR A SLIP JOINT TO LIMIT LOWEST POLE SEGMENT EXTENSION ABOVE GRADE TO 12'-0" MAXIMUM.

To maintain 12' section length for section A is not optimized solution. Is this applicable for base plated poles?

-This note does not apply to any structure.

2. Loading point- In the str 63, loading tree shows load point at point no. 8, but in the table, loads for the same are not given. Can you please provide the loading vectors for point no.8?



Point loads 7 should be applied to point load 8 location.

3. For Str 11, vectors loads for point 'L3' are not given in the loading table, but shown in the loading tree. Please provide.

LOADING TABLE							
LOAD	CASE 1	CASE 2	CASE 3	CASE 7	CASE 9		
V1	200	200	600	100	600		
T1	1100	900	1200	200	1100		
L1	-100	-100	-100	-100	-300		
V2	900	500	1300	400	1400		
T2	4300	3600	3400	1000	3100		
L2	-100	-100	-100	-100	-200		
V3	500	300	1000	300	1000		
Т3	3900	5800	2800	600	2300		
L3	-100	-100	-100	-100	-1300		
V4	400	200	900	200	900		
T4	2000	1700	1800	400	1600		
L4	-100	-100	-100	-100	-1000		
V5	300	100	800	100	900		
T5	900	1200	900	200	700		
L5	-100	-100	-100	-100	-700		
V6	500	200	1000	200	1100		
T6	900	1600	1000	100	700		
L6	-100	-100	-100	-	-600		
V7	500	200	1600	200	200		
T7	-1900	-900	-1500	-600	-1600		
L7	-400	-200	-300	-100	-300		
V8	500	200	1600	200	200		
Т8	-1900	-900	-1500	-600	-1600		
L8	-400	-200	-300	-100	-300		
W(PSF)	10	36.9	4.1	0	0		

Loading table for STR 11 is provided below.

4. Arm Detail – For structure no.23, the arm type detail is missing. Can you please provide the necessary arm type detail.

Should be detail AR5.5D for arm detail on STR 23.

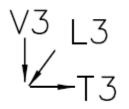
- In pages 25/63 (DE-30R\_STR-4) & 29/63 (DE-60R\_STR-6) V4,T4,L4 loads are provided twice in load table. We are planning to apply both loads at load point 4.
   Apply the point load that creates the most severe effect on the pole
- 6. In page 28/63 load point 8 is shown in both elevation and load tree but is not available in load table. We are not applying any load for this.

Apply point load 7 at both locations on load tree where it says apply point load 7 and point load 8.

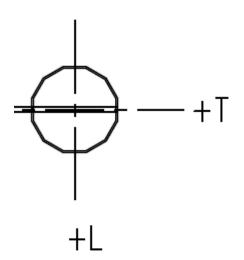
7. In page 32/63 Distribution arm detail is not specified. Assuming Detail "AR5.5D" for this. Should be detail AR5.5D for arm detail on STR 23.

 For few structures Distribution cross arm are by others for example in page 37/63 for which arm length is not available. Assuming 10ft for this for load offset purpose.
 All Running angle structures have 12' crossarms

In page 40/63 all transverse loads are applied in -Ve direction as per the line angle irrespective of the load tree.
 All point loads on the load tree should be as shown below, Positive down, Positive out of page, Positive right. Load directions in load table are correct.



Per PLS CADD default settings, Positive Longitudinal axis is always on back span face of pole and positive Transverse axis is always on the right side of the pole looking towards the ahead span.



- In page 42/63 all transverse loads are applied in +Ve direction as per the line angle irrespective of the load tree and load table except for T7,T8 which are applied in opposite to that of other loads similar to page 41.
   See answer to question 9.
- Some pole structures are with –ve vertical loads due to which slip joints are in uplift. Can we propose the slip joint with splice lock bars for such a cases? Or we have to provide the flange joint for such a cases. Please confirm.
   Per GUC, Please provide the flange joint for such a case.

- Customer provided RFQ document/package do not include drawings for WPE poles. We need drawings to provide appropriate hardware and to check interference if any. These drawings will be provided after award of Bid.
- 2. Customer provided structure information shows "vibratory steel caisson" for all WPE poles. Please confirm if these poles are to be direct embedded (24ft & 25ft as provided) or do we need to provide slip fit caisson (a 'zero-taper' section from certain height above GL which will be slipped with the top sections of WPE pole). If it is going to be slip fit caissons, we need detailed drawings shows reveal, ground sleeve and Corrocote information for reference. Ground sleeve details can be found in Section II.5.a.(8) of pole specification. Corrocote details can be found in Section II.5.f.(1).a of pole specification.
- 3. We are confused with the file name for .LCA files provided. For example, we have a lca with the name 10.11.lca and not sure if we have to use this .lca to design STR #10 or STR #11. The first number correlates to the structure number. So "10.11.lca" is the lca file for Str #10. Consistent throughout line
- In newly received addendum, there are 5 custom structures, STR # 8, 55, 56, 75 & 85 added which are not present in the customer provided drawings or in the latest document received. We are not sure that these newly added structures are belonging to which drawing. Below is the snapshot for your reference.

Will provide LCA and BAK files for engineered tangent structures. These structure drawings will be provided after award of bid as well.

5. We found al notes are related with vibratory caissons and embedded section, we are confused about it, will GUC use our embedded part(embedment depth) of pole as vibratory steel caissons, or embedded section? We have designed structure 2 for example, pole height is 85ft above ground level, and embedment depth 25ft, pole total length is 110ft, correct? If not correct, please help to clarify,

Structure 2 example is Correct. The pole height provided is for the above ground portion of the pole. The embedment depth is independent from the pole height.

#### SUBMIT BIDS ON ATTACHED PROPOSAL FORM

#### SPECIFICATIONS FOR TUBULAR STEEL STRUCTURES

In the space below, please provide a price quote for the following Tubular Steel Structures per the attached specifications. If your firm is unable to furnish this Tubular Steel Structures in accordance with our specifications and you wish to offer a substitute, please provide us with full details of the approved equivalent.

Item No.	Estimated Quantity	Description	Delivery Time	Unit Price	Extension
Ι	163	Tubular Steel Structures		\$	\$

Method of Award: Items I will be awarded as a total bid.

**Complete and Check All Math:** It is the responsibility of the Bidder to extend bid prices and supply a total for all items. It is certified that this proposal is made in good faith and without collusion or connection with any other person bidding on the same above listed items. It is also certified that this proposal is made in good faith and without collusion or connection with any GUC employee(s).

<u>The Undersigned Bidder</u>: hereby declares that it has carefully examined the enclosed detailed specifications for furnishing GUC with the below listed items. The undersigned bidder further agrees, if this proposal is accepted within thirty (30) days from the date of the opening, to furnish any or all of the items upon the quoted price.

[Balance of page left blank intentionally]

It is certified that this proposal is made in good faith and without collusion or connection with any other person bidding on the same above listed items. It is also certified that this proposal is made in good faith and without collusion or connection with any GUC employee(s).

Each Proposal shall be accompanied by cash, cashier's check, or certified check drawn on a bank insured with the Federal Deposit Insurance Corporation or the Savings Association Insurance Fund, payable to the Owner, in an amount not less than five percent (5%) of the total bid as a guarantee that a Purchase Order, if awarded, will be accepted. In lieu thereof, a Bid Bond may be submitted by the Bidder in an amount not less than five percent (5%) of the total bid (see attached Bid Bond form). The total bid price for which the five percent (5%) applies shall be the total of all schedules.

Certified check or cash for \$	or bid l	oond for <u>\$</u>	attached.
Firm Name		Pho	one ()
Address			
City	State		Zip Code
Fax ()	E-	Mail	
Authorized Official	Typed Name	Tit	le
	Signature	Da	te

#### BID BOND

KNOW ALL MEN BY THESE PRESENT, THAT WE

as Principal, and

as Surety, who is duly licensed to act as Surety in North Carolina, are held and firmly bound unto the <u>Greenville Utilities Commission of Greenville, North Carolina</u>, as Obligee, in the penal sum of \_\_\_\_\_\_

\_ DOLLARS (\$\_\_\_\_\_) (5%

Bid Bond), lawful money of the United States of America, for the payment of which, well and truly to be made, we bind ourselves, our heirs, executors, administrators, successors and assigns, jointly and severally, firmly by these present.

SIGNED, Sealed and dated this \_\_\_\_\_ day of \_\_\_\_\_, 2018.

WHEREAS, the said Principal is herewith submitting a Proposal for

#### **Tubular Steel Structures**

and the Principal desires to file this Bid Bond in lieu of making the cash deposit as required by the bidding documents contained herein;

NOW, THEREFORE, THE CONDITION OF THE ABOVE OBLIGATION is such that if the principal shall be awarded the Purchase Order for which the bid is submitted and shall accept the Purchase Order within ten (10) days after the award of same to the principal, then this obligation shall be null and void; but if the principal fails to so accept such Purchase Order as required by the bidding documents contained herein, the Surety shall, upon demand, forthwith pay to the Obligee the amount set forth in the first paragraph hereof, and upon failure to forthwith make such payment, the Surety shall pay the Obligee an amount equal to double the amount of this Bid Bond as set forth in the first paragraph hereof. Power of Attorney from the Surety to its Attorney-in-Fact is attached hereto.

	Principal	
Ву		(SEAL)

Corporate Surety

By\_\_\_\_\_(SEAL)

Three (3) hard copies and One (1) Electronic copy of your proposal should be received

No later than March 31, 2022 at 3:00 pm (EDST).

# <u>NO BIDS CONSIDERED UNLESS SUBMITTED ON THIS FORM(S)</u> <u>RETURN ONLY BID SECURITY, THIS FORM, COST FORM, EXCEPTION FORM,</u> <u>E-VERIFY FORM(S)</u>

[Balance of page left blank intentionally]

#### **GREENVILLE UTILITIES COMMISSION**

#### **EXCEPTION/VARIATION FORM**

#### FOR TUBULAR STEEL STRUCTURES

**Provider's Certification**: This is to certify that it is our intent to furnish equipment, material, services, etc. in absolute compliance with the bid specification except where expressly noted below.

**Instructions**: List all exceptions or variations to these bid specifications. Providers shall identify each exception or variation by specification page. The omission of exception or variation information shall be deemed by the Commission as the Provider's intent to absolutely comply with the bid specification. If additional space is required, Provider may reproduce this form as necessary.

Page #	Exception/Variation
	tification:
Address	

#### Letter of Compliance to E-Verify for Greenville Utilities Commission

- 1. I have submitted a bid for contract or desire to enter into a contract with the Greenville Utilities Commission;
- 2. As part of my duties and responsibilities pursuant to said bid and/or contract, I affirm that I am aware of and in compliance with the requirements of E-Verify, Article 2 of Chapter 64 of the North Carolina General Statutes, to include (mark which applies):
- 3. \_\_\_\_\_ After hiring an employee to work in the United States I verify the work authorization of said employee through E-Verify and retain the record of the verification of work authorization while the employee is employed and for one year thereafter; or
- 4. \_\_\_\_\_ I employ less than fifteen (15) employees in the State of North Carolina.
- 5. As part of my duties and responsibilities pursuant to said bid and/or contract, I affirm that to the best of my knowledge and subcontractors employed as a part of this bid and/or contract, are in compliance with the requirements of E-Verify, Article 2 of Chapter 64 of the North Carolina General Statutes, to include (mark which applies):
- 6. \_\_\_\_\_ After hiring an employee to work in the United States the subcontractor verifies the work authorization of said employee through E-Verify and retains the record of the verification of work authorization while the employee is employed and for one year thereafter; or
- 7. \_\_\_\_ Employ less than fifteen (15) employees in the State of North Carolina.

	Specify subcontractor:	
		(Company Name)
By:		(Typed Name)
		(Authorized Signatory)
-		(Title)
_		(Date)

#### **SECTION III**

#### TERMS AND CONDITIONS FOR THE PURCHASE OF

#### APPARATUS, SUPPLIES, MATERIALS, AND EQUIPMENT

These Terms and Conditions, made and entered into on this the \_\_\_\_\_ day of \_\_\_\_\_, by and between GREENVILLE UTILITIES COMMISSION OF THE CITY OF GREENVILLE, PITT COUNTY, NORTH CAROLINA, with one of its principal offices and places of business at 401 S. Greene Street, Post Office Box 1847, Greenville, Pitt County, North Carolina 27835-1847, hereinafter referred to as "GUC" and

\_\_\_\_\_\_, a \_\_\_\_\_ organized and existing under and by virtue of the laws of the State of \_\_\_\_\_\_, with one of its principal offices and places of business at

\_\_\_\_\_, hereinafter referred to as

#### "PROVIDER";

### 1.0 <u>TAXES</u>

No taxes shall be included in any bid prices. GUC is exempt from Federal Excise Tax. GUC is not exempt from North Carolina state sales and use tax or, if applicable, Pitt County sales and use tax. Such taxes shall be shown as a separate item on the invoice.

### 2.0 <u>INVOICES</u>

It is understood and agreed that orders will be shipped at the established contract prices and quantities in effect on dates orders are placed. Invoicing at variance with this provision may subject the contract to cancellation. Applicable North Carolina sales tax shall be invoiced as a separate line item. All invoices must bear the GUC purchase order number. Mail all invoices to Greenville Utilities Commission, Finance Department, P.O. Box 1847, Greenville, NC 27835-1847.

#### 3.0 PAYMENT TERMS

Payments for equipment, materials, or supplies will be made after the receipt and acceptance of the equipment, materials, supplies or services and after submission of a proper invoice. GUC's normal payment policy is thirty (30) days. GUC will not be responsible for any goods delivered without a purchase order having been issued. Payment will be made in U.S. currency only.

### 4.0 <u>QUANTITIES</u>

Quantities specified are only estimates of GUC's requirements. GUC reserves the right to purchase more or less than the stated quantities at prices indicated in the submitted Proposal Form based on our actual needs.

### 5.0 AFFIRMATIVE ACTION

The Provider will take affirmative action in complying with all Federal and State requirements concerning fair employment and employment of the handicapped, and concerning the treatment of all employees, without discrimination by reason of race, color, religion, sex, national origin, or physical handicap.

## 6.0 <u>CONDITION AND PACKAGING</u>

Unless otherwise indicated in the bid, it is understood and agreed that any item offered or shipped shall be new and in first class condition, that all containers shall be new and suitable for storage or shipment, and that prices include standard commercial packaging.

## 7.0 <u>SAMPLES</u>

Samples of items, if required, must be furnished free of expense to GUC, and if not destroyed, will, upon request, be returned at the Provider's expense. Request for the return of samples must be made at the bid opening; otherwise, the samples will become GUC's property. Each individual sample must be labeled with Provider's name.

### 8.0 <u>SPECIFICATIONS</u>

Any deviation from specifications must be clearly pointed out, otherwise, it will be considered that items offered are in strict compliance with specifications, and the Provider will be held responsible. Deviations must be explained in detail. **The Provider shall not construe this paragraph as inviting deviation or implying that any deviation will be acceptable.** 

#### 9.0 INFORMATION AND DESCRIPTIVE LITERATURE

Providers are to furnish all information requested. Further, as may be specified elsewhere, each Provider must submit with its proposal: cuts, sketches, descriptive literature, and/or complete specifications covering the products offered. Reference to literature submitted with a previous bid does not satisfy this provision. Bids which do not comply with these requirements will be subject to rejection.

### 10.0 AWARD OF CONTRACT

As directed by statute, qualified bids will be evaluated and acceptance made of the lowest responsible, responsive bid most advantageous to GUC as determined upon consideration of such factors as prices offered, the quality of the article(s) offered, the general reputation and performance capabilities of the Provider, substantial conformity with the specifications and other conditions set forth in the bid, the suitability of the article(s) for the intended use, the related services needed, the date(s) of delivery and performance, and such other factors deemed by GUC to be pertinent or peculiar to the purchase in question.

Acceptance of the order includes acceptance of all terms, conditions, prices, delivery instructions, and specifications as shown on this set of Terms and Conditions and in this order or attached to and made a part of this order.

The conditions of this order cannot be modified except by written amendment in the form of "Amended Purchase Order," which has been approved by GUC's Procurement Manager.

In the event of a Provider's failure to deliver or perform as specified, GUC reserves the right to cancel the order or any part thereof, without prejudice to GUC's other rights. The Provider agrees that GUC may return part of or all of any shipment at Provider's expense. GUC may charge the Provider with all reasonable expenses resulting from such failure to deliver or perform.

#### 11.0 MEDIATION/BINDING ARBITRATION

In the event of any dispute between the Parties, the Parties agree to submit any dispute to nonbinding mediation before a mutually agreeable Mediator prior to initiating litigation. If the Parties are unable to agree upon a Mediator within thirty (30) days after demand therefore, either Party may petition a Court of competent jurisdiction for the designation of a qualified Mediator for these purposes. Each Party shall bear its own costs and expenses of participating in the mediation (including, without limitation, reasonable attorneys' fees), and each Party shall bear one-half (1/2) of the costs and expenses of the Mediator. Unless otherwise agreed, the Parties will hold the mediation in Greenville, North Carolina. The matters discussed or revealed in the mediation session shall not be disclosed in any subsequent litigation.

In the event the matter is not resolved in mediation, either Party may request arbitration. The parties shall jointly select an Arbitrator, and shall be bound by the decision of the Arbitrator with respect to any dispute between the parties with respect to this Agreement. If the parties are unable to mutually agree upon an Arbitrator, the Parties shall each select an Arbitrator, and the two Arbitrators so selected shall select a third Arbitrator, and the decision of the majority of the Arbitrators shall be conclusive and binding upon the Parties. The Parties at all times agree to equally split the costs of any Arbitrator(s) selected in an effort to resolve the dispute between the Parties. Any party desiring to resolve a dispute under the terms of this Agreement shall notify the other Party in writing, and the Parties shall seek to agree upon a mutually agreed-upon Arbitrator within a period of ten (10) days from the date of such written demand. If the Parties are unable to agree within such ten (10) day period, the Parties shall each select an Arbitrator, and the two (2) Arbitrators so selected shall select a third Arbitrator within fifteen (15) days from the date of the written demand for arbitration, and a decision shall be rendered by the Arbitrator(s) so selected within five (5) days after such Arbitrator(s) is selected.

#### 12.0 GOVERNMENT RESTRICTIONS

In the event any Governmental restrictions may be imposed which would necessitate alteration of the material, quality, workmanship, or performance of the items offered on this bid prior to their delivery, it shall be the responsibility of the successful Provider to notify the GUC Procurement Manager, at once, indicating in its letter the specific regulation which required such alterations. GUC reserves the right to accept any such alterations, including any price adjustments occasioned thereby, or, in the sole discretion of GUC, to cancel the contract.

### 13.0 INSURANCE

**13.1** Coverage – During the term of the contract, the Provider at its sole cost and expense shall provide commercial insurance of such type and with the following coverage and limits:

**13.1.1 Workers' Compensation** – The Provider shall provide and maintain Workers' Compensation Insurance, as required by the laws of North Carolina, as well as employer's liability coverage with minimum limits of \$1,000,000 each accident, covering all Providers' employees who are engaged in any work under the contract. If any work is sublet, the Provider shall require the subcontractor to provide the same coverage for any of its employees engaged in any work under the contract.

- **13.1.2 General Liability** Commercial Liability Coverage written on an "occurrence" basis in the minimum amount of \$1,000,000 per occurrence.
- **13.1.3** Automobile Automobile Liability Insurance, to include coverage for all owned, hired, and non-owned vehicles used in connection with the contract with a minimum combined single limit of \$1,000,000 per accident.

**13.2 Requirements** - Providing and maintaining adequate insurance coverage is a material obligation of the Provider. All such insurance shall meet all laws of the State of North Carolina. Such insurance coverage shall be obtained from companies that are authorized to provide such coverage and that are authorized to do business in North Carolina by the Commissioner of Insurance. The Provider shall at all times comply with the terms of such insurance policies and all requirements of the insurer under any of such insurance policies, except as they may conflict with existing North Carolina laws or this contract. The limits of coverage under each insurance policy maintained by the Provider shall not be interpreted as limiting the Provider's liability and obligations under the contract. It is agreed that the coverage as stated shall not be canceled or changed until thirty (30) days after written notice of such termination or alteration has been sent by registered mail to GUC's Procurement Manager.

#### 14.0 PATENTS AND COPYRIGHTS

The Provider shall hold and save GUC, its officers, agents, and employees, harmless from liability of any kind, including costs and expenses, including reasonable attorney fees, on account of any copyrighted articles or any patented or unpatented invention, device or appliance manufactured or used in the performance of this contract.

#### 15.0 PATENT AND COPYRIGHT INDEMNITY

The Provider will defend or settle, at its own expense, any action brought against GUC to the extent that it is based on a claim that the product(s) provided pursuant to this agreement infringe any U.S. copyright or patent; and will pay those costs, damages, and attorney fees finally awarded against GUC in any such action attributable to any such claim, but such defense, settlements, and payments are conditioned on the following: (1) that Provider shall be notified promptly in writing by GUC of any such claim; (2) that Provider shall have sole control of the defense of any action on such claim and of all negotiations for its settlement or compromise; (3) that GUC shall cooperate with Provider in a reasonable way to facilitate the settlement of defense of such claim; (4) that such claim does not arise from GUC modifications not authorized by the Provider or from the use of combination of products provided by the Provider with products provided by GUC or by others; and (5) should such product(s) become, or in the Provider's opinion likely to become, the subject of such claim of infringement, then GUC shall permit Provider's option and expense, either to procure for GUC the right to continue using the product(s), or replace or modify the same so that it becomes non-infringing and performs in a substantially similar manner to the original product.

### 16.0 EXCEPTIONS

All proposals are subject to the terms and conditions outlined herein. All responses will be controlled by such terms and conditions and the submission of other terms and conditions, price catalogs, and other documents as part of a Provider's response will be waived and have no effect on this Request for Proposal or any other contract that may be awarded resulting from this solicitation. The submission of any other terms and conditions by a Provider may be grounds for rejection of the Provider's proposal. The Provider specifically agrees to the terms and conditions set forth in this set of Terms and Conditions by affixing its name on the signatory page contained herein.

#### 17.0 CONFIDENTIAL INFORMATION

Except as provided by statute and rule of law, GUC will keep trade secrets in which the Provider does not wish disclosed confidential. Each page shall be identified in boldface at the top and bottom as "CONFIDENTIAL" by the Provider. Cost information shall not be deemed confidential. The determination of whether a matter is confidential will be determined by North Carolina law.

#### 18.0 ASSIGNMENT

No assignment of the Provider's obligations or the Provider's right to receive payment hereunder shall be permitted without the express written consent of GUC, provided however, upon written request approved by the GUC Procurement Manager, solely as a convenience to the Provider, GUC may:

- Forward the Provider's payment check directly to any person or entity designated by the Provider, and
- Include any person or entity designated by Provider as a joint payee on the Provider's payment check.
- In no event shall such approval and action obligate GUC to anyone other than the Provider, and the Provider shall remain responsible for fulfillment of all contract obligations.

#### 19.0 ACCESS TO PERSON AND RECORDS

GUC shall have reasonable access to persons and records of Provider as a result of all contracts entered into by GUC.

#### 20.0 INSPECTION AT BIDDER'S SITE

GUC reserves the right to inspect, at a reasonable time, the item, plant, or other facilities of a prospective Provider prior to contract award and during the contract term as necessary for GUC's determination that such item, plant, or other facilities conform to the specifications/requirements and are adequate and suitable for the proper and effective performance of the contract. Provider may limit GUC's access to restricted areas.

## 21.0 AVAILABILITY OF FUNDS

Any and all payments of compensation of this specific transaction and any continuation or any renewal or extension are dependent upon and subject to the allocation of GUC funds for the purpose set forth in this Agreement.

### 22.0 GOVERNING LAWS

All contracts, transactions, agreements, etc., are made under and shall be governed by and construed in accordance with the laws of the State of North Carolina.

# 23.0 ADMINISTRATIVE CODE

Bids, proposals, and awards are subject to applicable provisions of the North Carolina Administrative Code and General Statues and Laws of the State of North Carolina.

# 24.0 EXECUTION

In the discretion of GUC, failure of a duly authorized official of Provider to sign the Signatory Page may render the bid invalid.

### 25.0 <u>CLARIFICATIONS/INTERPRETATIONS</u>

Any and all questions regarding these Terms and Conditions must be addressed to the GUC Procurement Manager. Do not contact the user directly. **These Terms and Conditions are a complete statement of the parties' agreement and may only be modified in writing signed by Provider and the GUC Procurement Manager.** 

### 26.0 <u>SITUS</u>

The place of all contracts, transactions, agreements, their situs and forum, shall be North Carolina, where all matters, whether in contract or tort, relating to the validity, construction, interpretation, and enforcement shall be determined.

# 27.0 TERMINATION OF AGREEMENT

GUC or Provider may terminate this Agreement for just cause at any time. Provider will be paid for all time and expenses incurred as of the termination date. Termination for just cause by either party shall be by certified letter and shall be effective thirty (30) days after signed and acknowledged receipt of said letter. Just cause shall be based on reasonable grounds, and there must be a fair and honest cause or reason for such action. The causes for termination, include, but are not limited to: (1) Provider's persistent failure to perform in accordance with the Terms and Conditions, (2) Provider's disregard of laws and regulations related to this transaction, and/or (3) Provider's substantial violation of the provisions of the Terms and Conditions.

### 28.0 <u>DELIVERY</u>

Shipments will be made only upon releases from a purchase order issued by GUC in accordance with GUC's current needs.

Time is of the essence with respect to all deliveries under this Agreement. Delivery of all equipment, materials, or supplies shall be made Free on Board (FOB) GUC Warehouse, 701 Utility Way, Greenville, North Carolina 27834, unless otherwise specified. The agreed price for such equipment, materials, or supplies shall include all costs of delivery and ownership, and risks of loss shall not be transferred from Provider to GUC until express written acceptance of delivery and inspection by GUC. Delivery hours are between 8:00 AM and 4:30 PM Tuesday-Thursday only. **GUC's purchase order number is to be shown on the packing slip or any related documents.** GUC reserves the right to refuse or return any delivery with no purchase order number or which is damaged. GUC will not be charged a restocking fee for any delivery which is refused or returned.

### 29.0 INDEMNITY PROVISION

Provider agrees to indemnify and save GREENVILLE UTILITIES COMMISSION of the City of Greenville, Pitt County, North Carolina, and the City of Greenville, North Carolina, its coowners, joint venturers, agents, employees, and insurance carriers harmless from any and all losses, claims, actions, costs, expenses including reasonable attorney fees, judgments, subrogations, or other damages resulting from injury to any person (including injury resulting in death), or damage (including loss or destruction) to property of whatsoever nature of any person arising out of or incident to the performance of the terms of this Contract by Provider, including, but not limited to, Provider's employees, agents, subcontractors, and others designated by Provider to perform work or services in, about, or attendant to, the work and services under the terms of this Contract. Provider shall not be held responsible for any losses, expenses, claims, subrogations, actions, costs, judgments, or other damages, directly, solely, and proximately caused by the negligence of Greenville Utilities Commission of the City of Greenville, Pitt County, North Carolina. Insurance covering this indemnity agreement by the Provider in favor of Greenville Utilities Commission of the City of Greenville, Pitt County, North Carolina, and the City of Greenville, North Carolina, shall be provided by Provider.

#### **30.0 FORCE MAJEURE**

Neither party shall be considered in default in the performance of its obligations hereunder to the extent that the performance of any such obligation is prevented or delayed by any cause, existing or future, which is beyond the reasonable control of such party. In any such event of force majeure, the parties shall advise each other of such event, and the parties shall negotiate an equitable adjustment to their respective obligations under this Agreement.

#### 31.0 WARRANTY(IES)

The Provider hereby includes all warranties, whether expressed or implied, including, but not limited to, the Implied Warranty of Merchantability and the Implied Warranty of Fitness for a Particular Purpose.

#### 32.0 INTEGRATED CONTRACT

These Terms and Conditions, Instructions to Bidders, Specifications, and the selected Provider's bid represents the entire contract between the Parties. No verbal or other written agreement(s) shall be held to vary the provisions of this Agreement.

### 33.0 CONTRACT PROVISIONS

Each of the provisions of these Terms and Conditions shall apply to the full extent permitted by law, and the invalidity in whole or in part of any provision shall not affect the remainder of such provision or any other provisions.

### 34.0 <u>E-VERIFY</u>

E-Verify - I understand that E-Verify is the federal E-Verify program operated by the United States Department of Homeland Security and other federal agencies, or any successor or equivalent program used to verify the work authorization of newly hired employees pursuant to federal law in accordance with NCGS §64-25 et seq. I am aware of and in compliance with the requirements of E-Verify and Article 2 of Chapter 64 of the North Carolina General Statutes. To the best of my knowledge, any subcontractors employed by me as a part of this contract are in compliance with the requirements of E-Verify and Article 2 of Chapter 64 of the North Carolina General Statutes.

#### 35.0 IRAN DIVESTMENT ACT CERTIFICATION

By acceptance of this purchase order, Vendor/Contractor certifies that, as of the date of the purchase order or contract, it is not on the Final Divestment List as created by the State Treasurer pursuant to N.C.G.S. § 143-6A-4. In compliance with the requirements of the Iran Divestment Act and N.C.G.S. § 143C-6A-5(b), Vendor/Contractor shall not utilize in the performance of the contract any subcontractor that is identified on the Final Divestment List.

#### 36.0 UNIFORM GUIDANCE

Contracts funded with federal grant or loan funds must be procured in a manner that conforms with all applicable federal laws, policies, and standards, including those under the Uniform Guidance (2 C.F.R. Part 200).

#### 37.0 <u>NOTICES</u>

Notices to the Parties should be sent to the names and addresses specified below:

Cleve Haddock, CLGPO Procurement Manager Greenville Utilities Commission P.O. Box 1847 Greenville, NC 27835-1847

Vendor Specified on Page 1 of Section III when awarded.

#### GREENVILLE UTILITIES COMMISSION

COMPANY NAME:

By:	Ву:
Anthony C. Cannon	Name (Print):
Title: <u>General Manager/CEO</u> (Authorized Signatory)	Title:(Authorized Signatory)
Date:	Date:
Attest:	Attest:
Name (Print): <u>Amy Wade</u>	Name (Print):
Title: Executive Secretary	Title: Corporate Secretary
Date:	Date:
(OFFICIAL SEAL)	(CORP. SEAL)

This instrument has been pre-audited in the manner required by the Local Government Budget and Fiscal Control Act.

By: Jeff W. McCauley

Title: Chief Financial Officer

Date: \_\_\_\_\_

APPROVED AS TO FORM AND LEGAL CONTENT:

\_\_\_\_\_

By:

Phillip R. Dixon

Title: <u>General Counsel</u>

Date: \_\_\_\_\_